

## Problem set 9

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### Problem 1

Use the Rayleigh minimization principle to determine the natural frequencies of a free-free shaft in torsion, assuming the structure is discretized using a single linear finite element. The strong form is given by

$$\frac{\partial}{\partial x} \left( G I_p \frac{\partial}{\partial x} \varphi(x, t) \right) = \rho I_p \ddot{\varphi}(x, t)$$

where the shaft parameters are:

- length  $\ell$ ,
- polar moment of inertia  $I_p$ ,
- shear modulus  $G$ ,
- mass density  $\rho$ .

The unknown  $\varphi$  represent the torsional vibration: angular displacement (twist angle) at position  $x \in [0, \ell]$  and time  $t \in [0, T]$ . At  $x = 0$  and  $x = \ell$  there are no external applied moments, this translates into natural boundary conditions:

$$\frac{\partial}{\partial x} \varphi(0, t) = \frac{\partial}{\partial x} \varphi(\ell, t) = 0 \quad \forall t \in ]0, T[.$$

### Problem 2

Develop a MATLAB script to compute the first two natural frequencies and the corresponding mode shapes associated with the longitudinal vibrations of a uniform free-free bar, using the subspace iteration method. The structure is characterized by the following physical parameters:

- length:  $\ell = 0.5$  m,
- cross-sectional area:  $A = 4 \cdot 10^{-4}$  m<sup>2</sup>,
- Young's modulus:  $E = 210$  GPa,
- mass density:  $\rho = 7850$  kg/m<sup>3</sup>.

The bar is modeled using a single quadratic finite element with three equally spaced nodes. The structure stiffness and lumped mass<sup>1</sup> matrices are given by:

$$\mathbf{K} = \frac{EA}{3\ell} \begin{bmatrix} 7 & -8 & 1 \\ -8 & 16 & -8 \\ 1 & -8 & 7 \end{bmatrix} \quad \text{and} \quad \mathbf{M} = \frac{\rho A \ell}{6} \text{diag}(1, 4, 1).$$

- 1) Formulate the generalized eigenvalue problem for the system, and define the stiffness and mass matrices explicitly.

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<sup>1</sup>The use of a lumped mass matrix is often justified by its ability to simplify computations, improve numerical stability in explicit time integration schemes, and avoid non-physical oscillations associated with consistent mass formulations in low-order elements.

2) Since the system exhibits a rigid-body mode with zero frequency, a spectral shift is applied to ensure convergence. The modified eigenvalue problem is:

$$(\mathbf{K} + \sigma \mathbf{M})\mathbf{p} = (\lambda + \sigma)\mathbf{M}\mathbf{p}$$

where the shift parameter is chosen (to simplify the computation) as:

$$\sigma = 2 \frac{E}{\rho \ell^2}.$$

Define the shifted stiffness matrix as  $\mathbf{K}_\sigma = \mathbf{K} + \sigma \mathbf{M}$ , yielding the equivalent eigenvalue problem:

$$\mathbf{K}_\sigma \mathbf{p} = \lambda_\sigma \mathbf{M}\mathbf{p}, \quad \text{with} \quad \lambda_\sigma = \lambda + \sigma.$$

3) Implement three iterations of the subspace iteration algorithm to approximate the two lowest eigenpairs  $(\lambda_i, \mathbf{p}_i)$ . For simplicity use the function `eig` to solve the reduced Rayleigh's minimization problem. Use the following initial guess:

$$\mathbf{P}_{(0)} = \frac{1}{\sqrt{\rho A \ell}} \begin{bmatrix} 1 & 0 \\ 0 & 1 \\ 0 & 0 \end{bmatrix}.$$

4) Compare the numerical approximation with the exact (shifted) eigenvalues and corresponding eigenvectors (defined up to a multiplicative constant):

$$\lambda_{\sigma,1} = 2 \frac{E}{\rho \ell^2} \quad \text{and} \quad \lambda_{\sigma,2} = 14 \frac{E}{\rho \ell^2},$$

$$\mathbf{p}_1 = \begin{bmatrix} -1 \\ -1 \\ -1 \end{bmatrix} \quad \text{and} \quad \mathbf{p}_2 = \begin{bmatrix} -1 \\ 0 \\ 1 \end{bmatrix}$$

and evaluate the accuracy by computing:

- The relative error between approximated and exact eigenvalues.
- The error in the  $\mathbf{M}$ -norm between approximated and exact eigenvectors.