

ME 470: Mechanics of Soft and biological matter.

LECTURE 6: 2D NETWORK - REGULAR TRIANGULAR LATTICE

CONTENTS

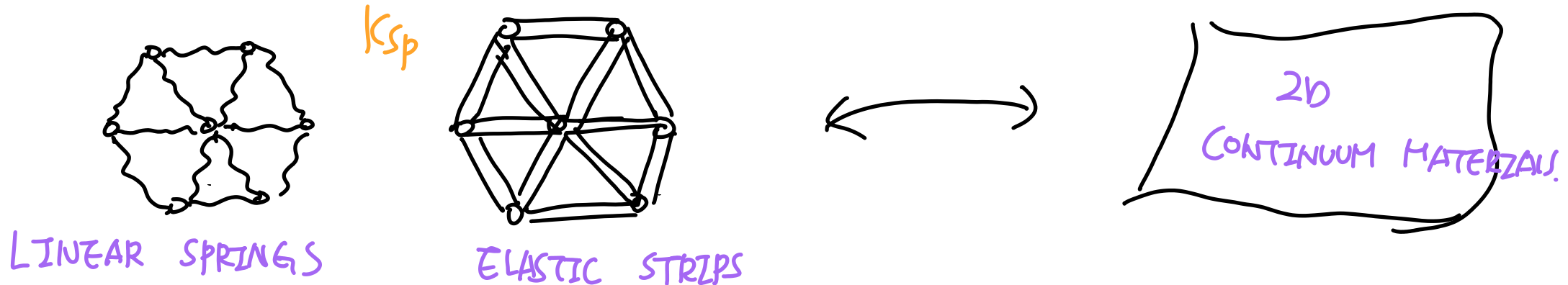
- 2D ELASTIC MODULUS OF REGULAR TRIANGULAR LATTICE
- REGULAR TRIANGULAR LATTICE.

3.3. STRETCHING IN SPRING NETWORK.

FOR A GENERAL NETWORK STRUCTURE, MECHANICAL PROPERTIES DEPEND ON MULTIPLE FACTORS SUCH AS.

- ① MECHANICAL PROPERTIES OF A SINGLE 1D ELEMENT
- ② ORIENTATION OF 1D ELEMENT
- ③ NUMBER OF 1D ELEMENTS IN A UNIT AREA
- ④ CROSS-LINK BETWEEN 1D ELEMENTS.

HERE, WE CONSIDER A VERY SIMPLE CASE.



INTERPRET REGULAR TRIANGULAR SPRING NETWORK AS 2D LINEAR ELASTIC MATERIALS.

⇒ CONSTITUTIVE RELATION: $\tau_{ij} = C_{ijkl} \epsilon_{kl}$, $i, j, k, l \in \{x, y\}$

NOT 16 UNIQUE COEFFICIENTS.

SYMMETRIES, $C_{ijkl} = C_{jikl} = C_{ijlk}$

THERMODYNAMICS $C_{ijkl} = C_{klij}$

⇒ LEAVES ONLY C_{xxxx} , C_{xxxy} , C_{xxyy} , C_{xyxy} , C_{yyyy} , C_{yyyz}

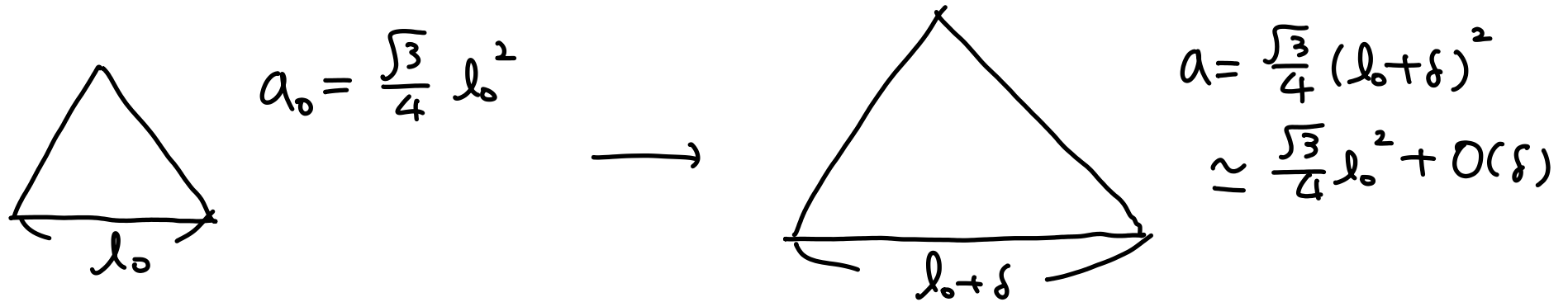
⇒ 6-FOLD SYMMETRY ONLY LEAVES 2 INDEPENDENT COEFFICIENTS.

TRIANGULAR SPRING NETWORK AS 2D ISOTROPIC HOMOGENEOUS ELASTIC MATERIALS.!

3.3.1. BULK MODULUS.

DUE TO SYMMETRY, WE ONLY NEED TO CONSIDER A SINGLE TRIANGLE UNIT, "PLAQUETTE"

⇒ UNDER EQUIBIAXIAL EXTENSION:



⇒ ENERGY PER PLAQUETTE: 2 PLAQUETTE SHARE 1 SPRING.

$$U = \underset{\substack{\uparrow \\ 3 \text{ SPRINGS PER PLAQUETTE}}}{3} \cdot \frac{1}{2} k_{sp} \delta^2 \cdot \underset{\substack{\downarrow \\ 2 \text{ PLAQUETTE SHARE 1 SPRING}}}{\left(\frac{1}{2}\right)} = \frac{3}{4} k_{sp} \cdot \delta^2$$

3 SPRINGS PER PLAQUETTE

ENERGY PER AREA:

$$u_A = \frac{U}{a_0} = \sqrt{3} k_{sp} \left(\frac{f}{l_0} \right)^2 \quad \text{--- ①}$$

STRAIN ENERGY DENSITY:

$$\left[\epsilon_{xx}?, \quad \epsilon_{xx} = \epsilon_{yy} = \frac{f}{l_0}, \quad \epsilon_{xy} = 0 \right]$$

$$u_A = \mu_A \left[2\epsilon_{xy}^2 + \frac{1}{2} (\epsilon_{xx} - \epsilon_{yy})^2 \right] + \frac{k_A}{2} (\epsilon_{xx} + \epsilon_{yy})^2$$

$$\Rightarrow u_A = \frac{k_A}{2} (\epsilon_{xx} + \epsilon_{yy})^2 = 2k_A \cdot \left(\frac{f}{l_0} \right)^2 \quad \text{--- ②}$$

EQUIBIAXIAL EXTENSION.

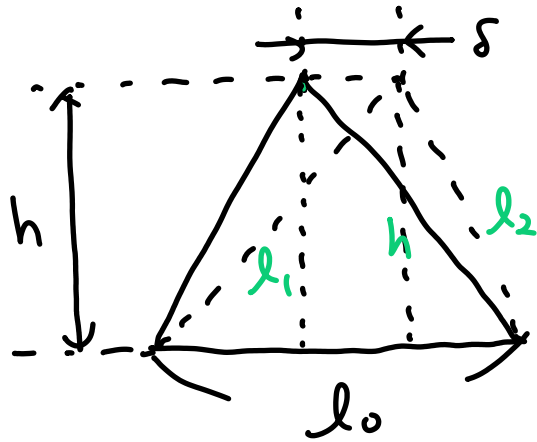
$$\text{①} = \text{②}$$

$\Rightarrow$

$$k_A = \frac{\sqrt{3}}{2} k_{sp}$$

3.3.2. SHEAR MODULUS.

SIMPLE SHEAR



$$l_1^2 = \left(\frac{l_0}{2} + \delta\right)^2 + \frac{3}{4}l_0^2 \quad \text{AND STILL } a=a_0 = \frac{\sqrt{3}}{4}l_0^2$$

$$(\text{FOR SMALL } \delta) \Rightarrow l_1 \simeq l_0 + \frac{\delta}{2}$$

$$l_2 \simeq l_0 - \frac{\delta}{2}$$

ENERGY PER PLAQUETTE

$$U = 2 \cdot \frac{1}{2} k_{sp} \cdot \left(\frac{\delta}{2}\right)^2 \cdot \frac{1}{2} = \frac{k_{sp}}{8} \delta^2$$

$$\Rightarrow U_A = \frac{U}{a_0} = \frac{k_{sp}}{2\sqrt{3}} \cdot \left(\frac{\delta}{l_0}\right)^2 \quad \text{--- (3)}$$

STRAIN ENERGY DENSITY.

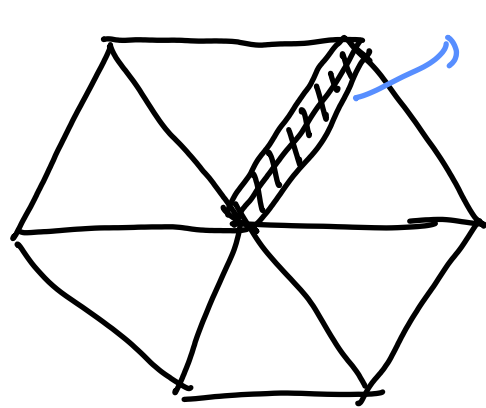
$$\epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \right) = \frac{1}{2} \cdot \frac{\delta}{h} = \frac{\delta}{\sqrt{3}l_0}$$

$$U_A = \mu_A \cdot 2 \cdot \left(\frac{\delta}{\sqrt{3}l_0}\right)^2 = \frac{2}{3} \mu_A \left(\frac{\delta}{l_0}\right)^2 \quad \text{--- (4)}$$

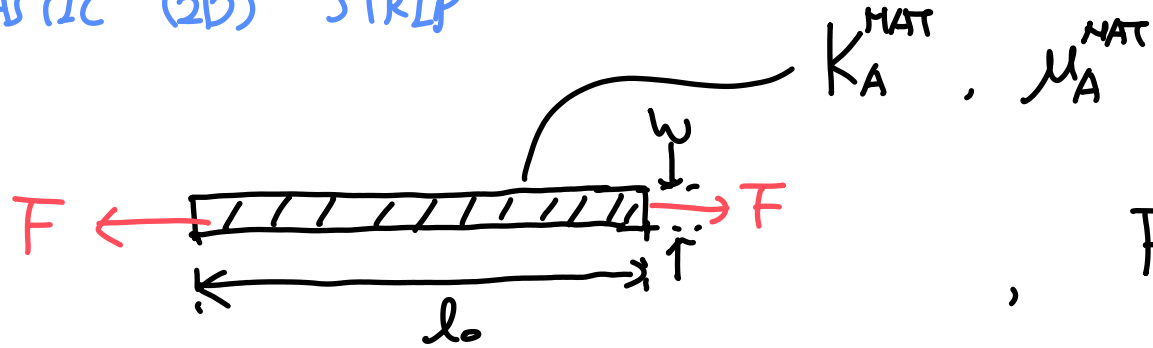
(3) = (4) YIELD

$$\mu_A = \frac{\sqrt{3}}{4} k_{sp} = \frac{1}{2} k_A$$

3.3.3. ELASTIC STRIPS



ELASTIC (2D) STRIP



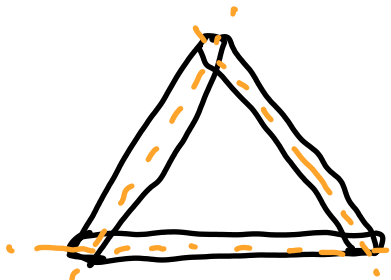
$$F = k_{sp} \cdot \delta$$

UNIAXIAL TENSION: $\tau_{11} = E_A^{MAT} \cdot \epsilon_{11}$

(WE ALREADY COMPUTED $E_A^{MAT} = 2 K_A^{MAT} \cdot (1 - \nu_A^{MAT})$)

$$\tau_{11} = \frac{F}{w} = k_{sp} \cdot \left(\frac{\delta}{w} \right) = k_{sp} \cdot \left(\frac{l_0}{w} \right) \cdot \underbrace{\left(\frac{\delta}{l_0} \right)}_{\epsilon_{11}}$$

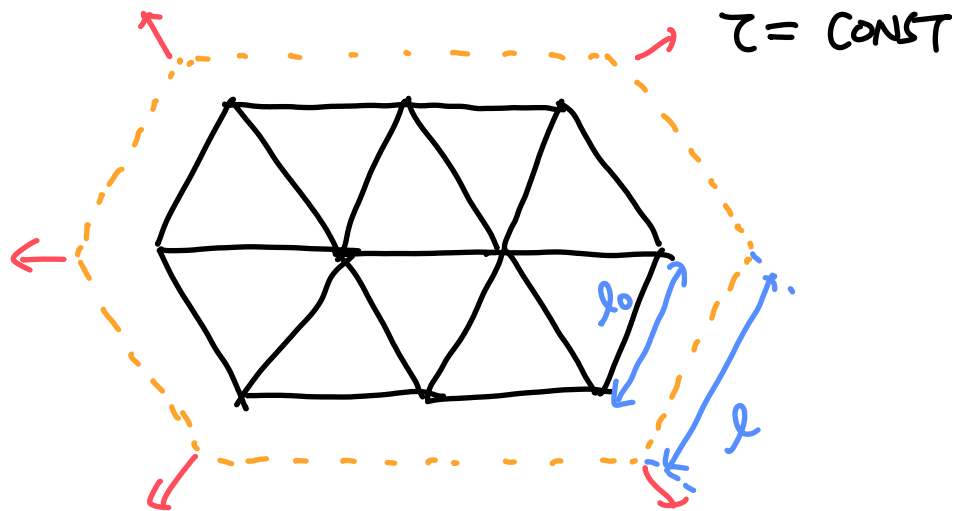
$$\Rightarrow E_A^{MAT} = k_{sp} \cdot \left(\frac{l_0}{w} \right) = \frac{2}{\sqrt{3}} K_A \cdot \left(\frac{l_0}{w} \right) = 2 K_A^{MAT} (1 - \nu_A^{MAT}) \Rightarrow \boxed{K_A = \sqrt{3} \cdot \frac{w}{l_0} \cdot (1 - \nu_A^{MAT}) K_A^{MAT}}$$



AREA FRACTION OF STRIPS?

$$\phi = \frac{3 \cdot w \cdot l_0}{\frac{\sqrt{3}}{4} l_0^2} \cdot \left(\frac{1}{2} \right) = 2\sqrt{3} \cdot \frac{w}{l_0} \Rightarrow \boxed{K_A = \frac{\phi}{2} \cdot (1 - \nu_A^{MAT}) K_A^{MAT}}$$

3.4. NETWORK UNDER EXTERNAL STRESS



SIMPLEST: $\tau > 0$

PER PLAQUETTE:

CONSIDER ENTHALPY.

$$(\delta = l - l_0)$$

$$U = \frac{3}{4} k_{sp} \delta^2$$

$$U_A = \sqrt{3} k_{sp} \left(\frac{\delta}{l_0} \right)^2$$

$$h_A = U_A - \tau_{ij} \epsilon_{ij}$$

$$H = U - \int_{\text{PLAQUETTE}} \tau_{ij} \epsilon_{ij} dA$$

HERE: $\tau_{ij} \epsilon_{ij} = \tau \cdot (\epsilon_{xx} + \epsilon_{yy}) = \tau \cdot \frac{\Delta a}{a_0} \quad (\Delta a = a - a_0)$

$$\Rightarrow H = U - \tau \Delta a = U - \tau a + \tau a_0$$

CONSTANT. NEGLECT

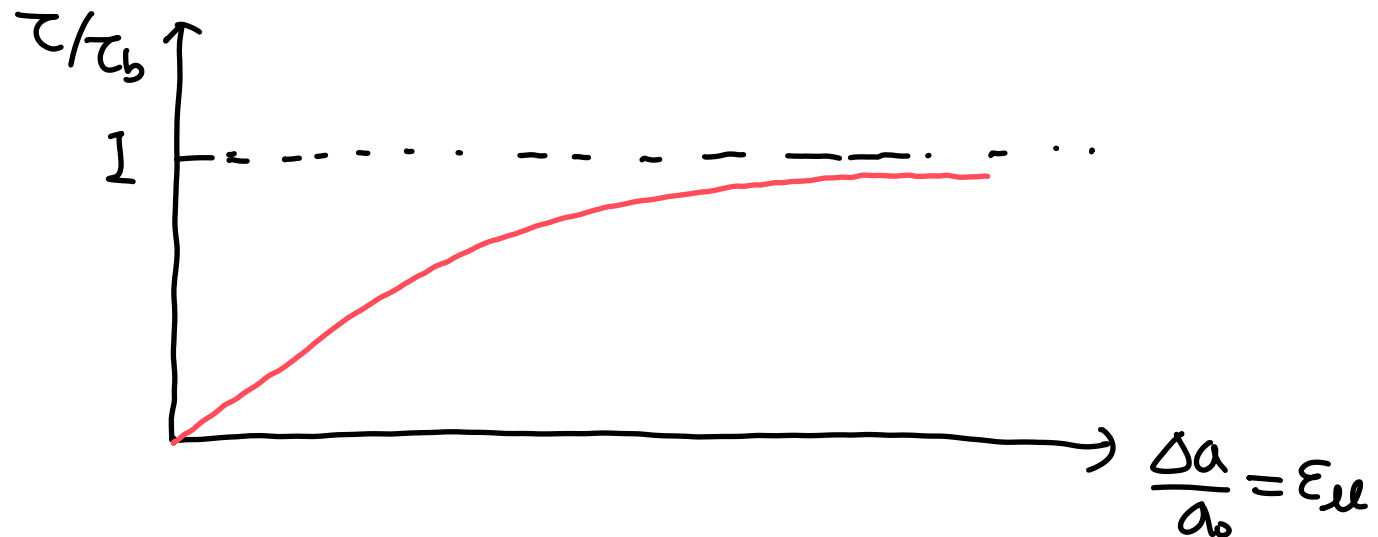
$$H = \frac{3}{4} k_{sp} (l - l_0)^2 - \tau \cdot \frac{\sqrt{3}}{4} l^2$$

EQUILIBRIUM: $dH=0 \iff \frac{dH}{dl}=0$ (ALL EQUILATERAL $\triangle$ ONLY, 1 VARIABLE)

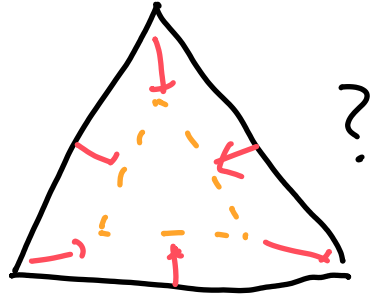
$$\frac{3}{2} k_{sp} (l - l_0) = \tau \cdot \frac{\sqrt{3}}{2} l$$

$$\Rightarrow \boxed{l = \frac{l_0}{1 - \tau/\tau_b}} \quad \text{WHERE } \boxed{\tau_b = \sqrt{3} k_{sp}}$$

CATASTROPHIC BLOW-UP ($l \rightarrow \infty$) AS $\tau \rightarrow \tau_b$



WHAT IF $\tau < 0$ (STILL UNIFORM)?



BUT, DEFORMATION CAN BE NON-UNIFORM.

$\Rightarrow$ HUGE # OF VARIABLES l_i IN H

$\Rightarrow$ NEED ALL $\frac{\partial H}{\partial l_i} = 0 \quad \forall i$

TO SOLVE THIS PROBLEM..

(1) NUMERICAL SOLUTION.

(2) CALCULUS OF VARIATION.

(3) MODE ANALYSIS

$\hookrightarrow$ PICK DISTINCTIVE MODES OF DEFORMATION, COMPUTE ENERGY FUNCTIONAL VALUES.

MODE 1

UNIFORM CONTRACTION.

$$H_u = \frac{3}{4} k_{sp} (l - l_0)^2 - \tau \cdot \frac{\sqrt{3}}{4} l^2 \quad (\text{SAME AS EXTENSION})$$

$$H_u^{eq}(\tau) = \frac{3}{4} k_{sp} l_0^2 \left(1 - \frac{1}{1 - \tau/\tau_b} \right) - \tau \cdot \frac{\sqrt{3}}{4} \cdot \frac{l_0^2}{(1 - \tau/\tau_b)^2}$$

MODE 2

PLAQUETTE COLLAPSE ($a \rightarrow 0$)

$$\triangle \Rightarrow \frac{l_2 \quad l_3}{2l_1} \quad 2l_1 = l_2 + l_3$$

ASSUMPTION: ALL PLAQUETTES COLLAPSE THE SAME WAY

$$H_c = U - \cancel{\tau a}^0 = \frac{1}{2} k_{sp} [(2l_1 - l_0)^2 + (l_2 - l_0)^2 + (2l_1 - l_2 - l_0)^2] \cdot \frac{1}{2}$$

$$dH_c = 0 \quad \Leftrightarrow \quad \frac{\partial H_c}{\partial l_1} = 0 \quad \text{AND} \quad \frac{\partial H_c}{\partial l_2} = 0$$

$$\frac{\partial H_c}{\partial l_2} = \frac{1}{4} K_{sp} [2(l_2 - l_0) - 2(2l_1 - l_2 - l_0)] = 0$$

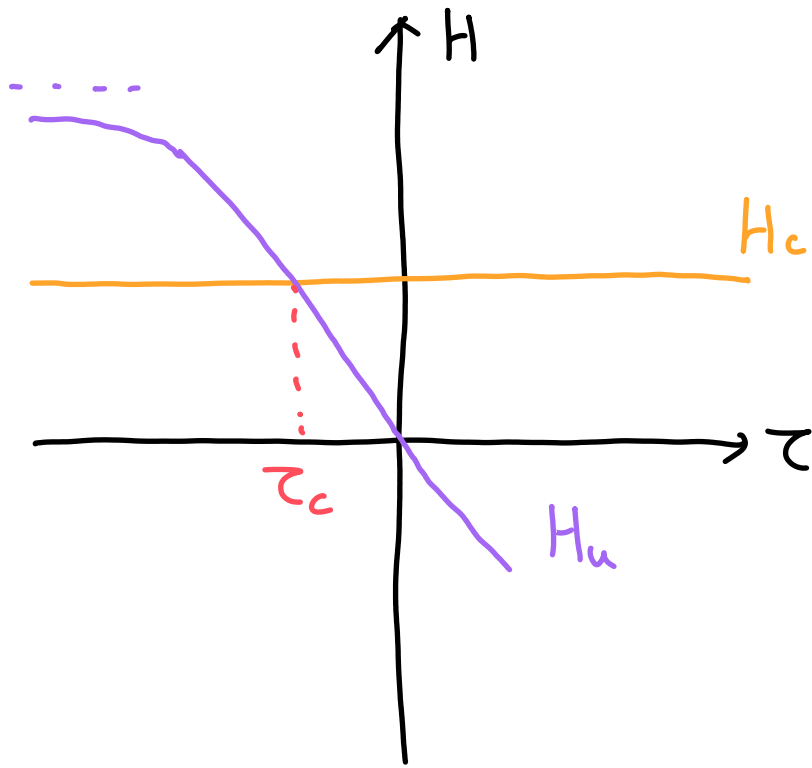
$$\frac{K_{sp}}{4} [4l_2 - 4l_1] = 0 \quad \Rightarrow \quad \boxed{l_1 = l_2}$$

$$\Rightarrow H_c = \frac{K_{sp}}{4} [(2l_1 - l_0)^2 + 2(l_1 - l_0)^2]$$

$$\frac{\partial H_c}{\partial l_1} = \frac{K_{sp}}{4} [2 \cdot (2l_1 - l_0) + 4(l_1 - l_0)]$$

$$= K_{sp} [3l_1 - 2l_0] = 0 \quad \Rightarrow \quad l_1 = \frac{2}{3} l_0$$

$$\Rightarrow H_c^{eq} = \frac{K_{sp}}{4} \left[\frac{l_0^2}{9} + 2 \cdot \frac{l_0^2}{9} \right] = \boxed{\frac{1}{12} K_{sp} l_0^2}$$



$$H_c^{eq} = H_u^{eq}$$

$$\Rightarrow \frac{1}{12} k_{sp} l_0 = \frac{3}{4} k_{sp} l_0 \left(1 - \frac{1}{1 - \tau/\tau_b}\right)^2 - \tau \cdot \frac{\sqrt{3}}{4} \cdot \frac{l_0}{(1 - \tau/\tau_b)^2}$$

$$1 = 9 \left(1 - \frac{1}{1 - \tau/\tau_b}\right)^2 - \frac{9\tau}{\sqrt{3} k_{sp}} \cdot \frac{1}{(1 - \tau/\tau_b)^2}$$

$\lambda = \tau/\tau_b$

$$1 = 9 \cdot \left(\frac{-\lambda}{1 - \lambda}\right)^2 - \frac{9\lambda}{(1 - \lambda)^2}$$

$$(1 - \lambda)^2 = 9\lambda^2 - 9\lambda$$

$$8\lambda^2 - 7\lambda - 1 = 0$$

$$(8\lambda + 1)(\lambda - 1) = 0$$

$$\Rightarrow \lambda = \begin{cases} -1/8 \\ 1 \end{cases}$$

$$\tau_c = -\frac{1}{8} \tau_b$$

NOTE:

$$l(\tau_c) = \frac{l_0}{1 - \tau_c/\tau_b} = \frac{8}{9} l_0 \quad (\text{STILL UNIFORM})$$

NOTE:

- ① ONLY 2 MODES ANALYZED, OTHERS MAY HAVE $H < H_u$ FOR $\tau > \tau_c$, AND LEAD TO INSTABILITY FIRST.
- ② NETWORK MUST DEFORM FROM U TO C THROUGH INTERMEDIATE STATES, THESE MIGHT HAVE HIGHER H^*

SUMMARY)

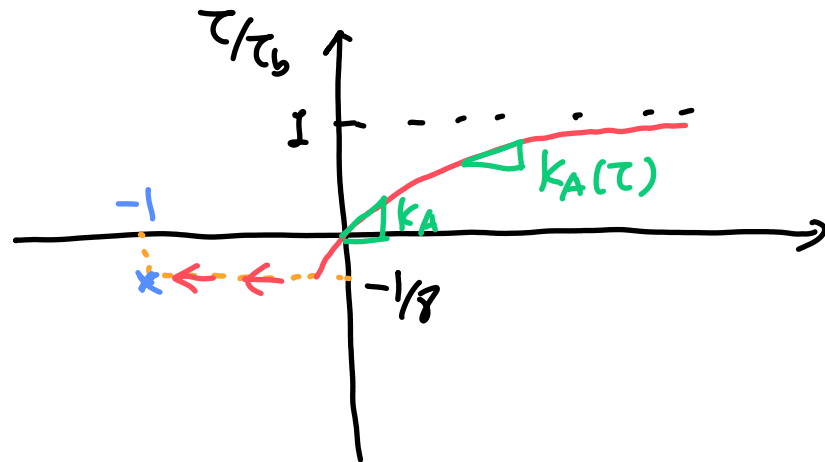
REGULAR TRIANGULAR NETWORK.

$$\left. \begin{aligned} K_A &= \frac{\sqrt{3}}{2} k_{sp} \\ \mu_A &= \frac{\sqrt{3}}{4} k_{sp} \end{aligned} \right\} \text{LINEAR.}$$

ENERGY ANALYSIS. (UNIFORM TENSILE STRESS, $\tau > 0$)

$$l(\tau) = \frac{l_0}{1 - \tau/\tau_b}, \quad \tau_b = \sqrt{3} k_{sp}$$

MODE ANALYSIS ($\tau < 0$) $H_c < H_u$ FOR $\tau < -\frac{1}{8} \tau_b$



$$\epsilon_{ee} = \frac{\Delta a}{a_0} = \frac{a - a_0}{a_0}$$



