

Precast Elements Made of UHPC

– From Research to Application –

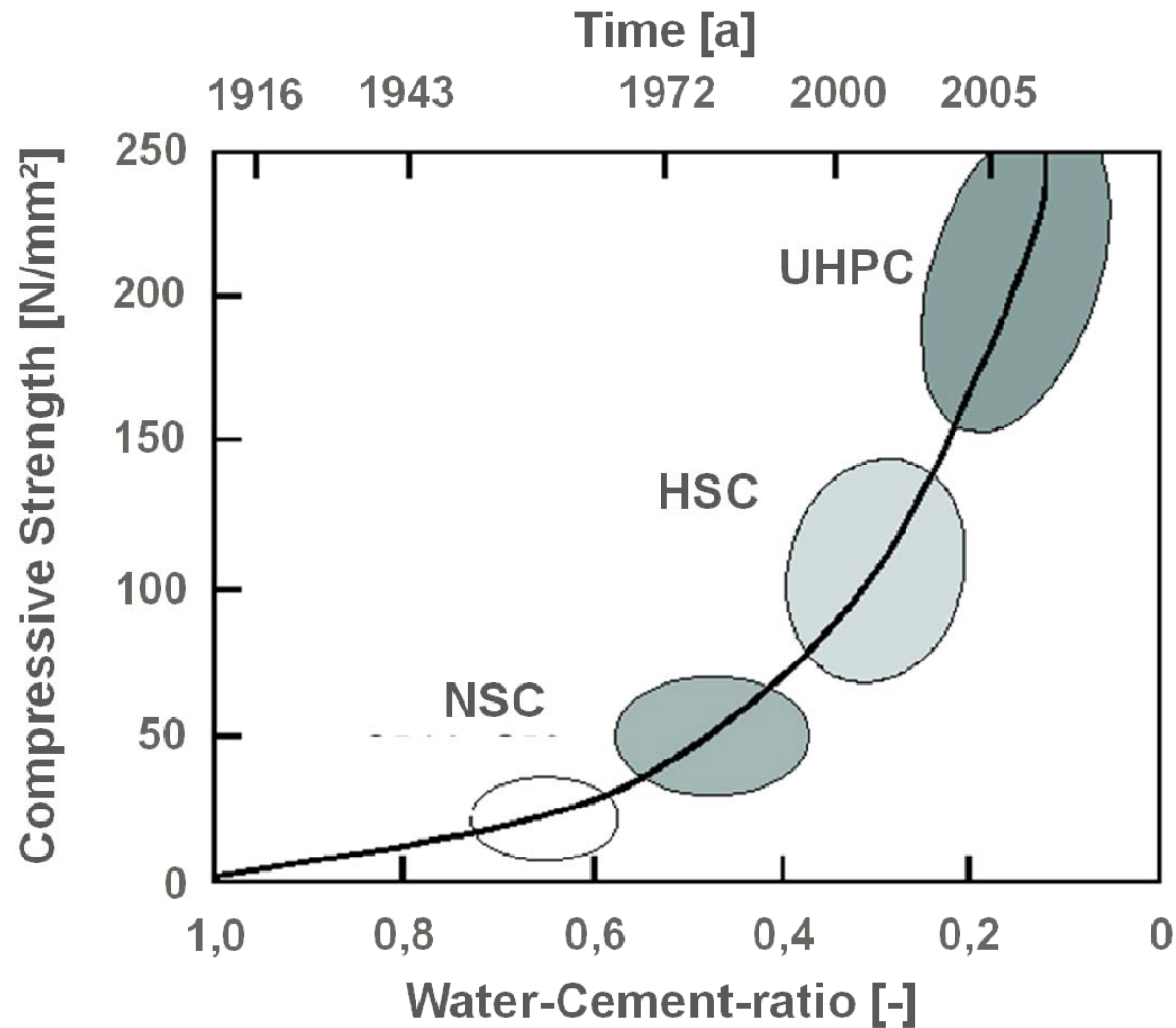


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Institute for Structural Concrete

Development of Concrete Compressive Strength



Ultra High Performance Concrete (UHPC)

A High-Tech Concrete

- Very dense Microstructure
- High workability
- High compressive strength (150-230MPa)
- High durability
-

Composition of UHPC

PCE -
superplasticizer



Silica fume



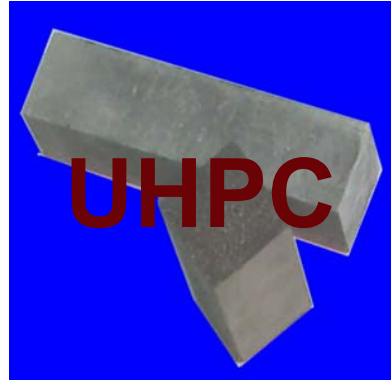
Quartz powder



water



UHPC



Micro steel fibre



cement



Basalt gravel



Quartz sand



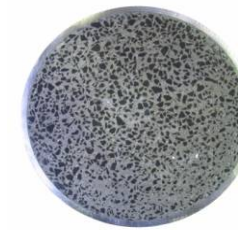
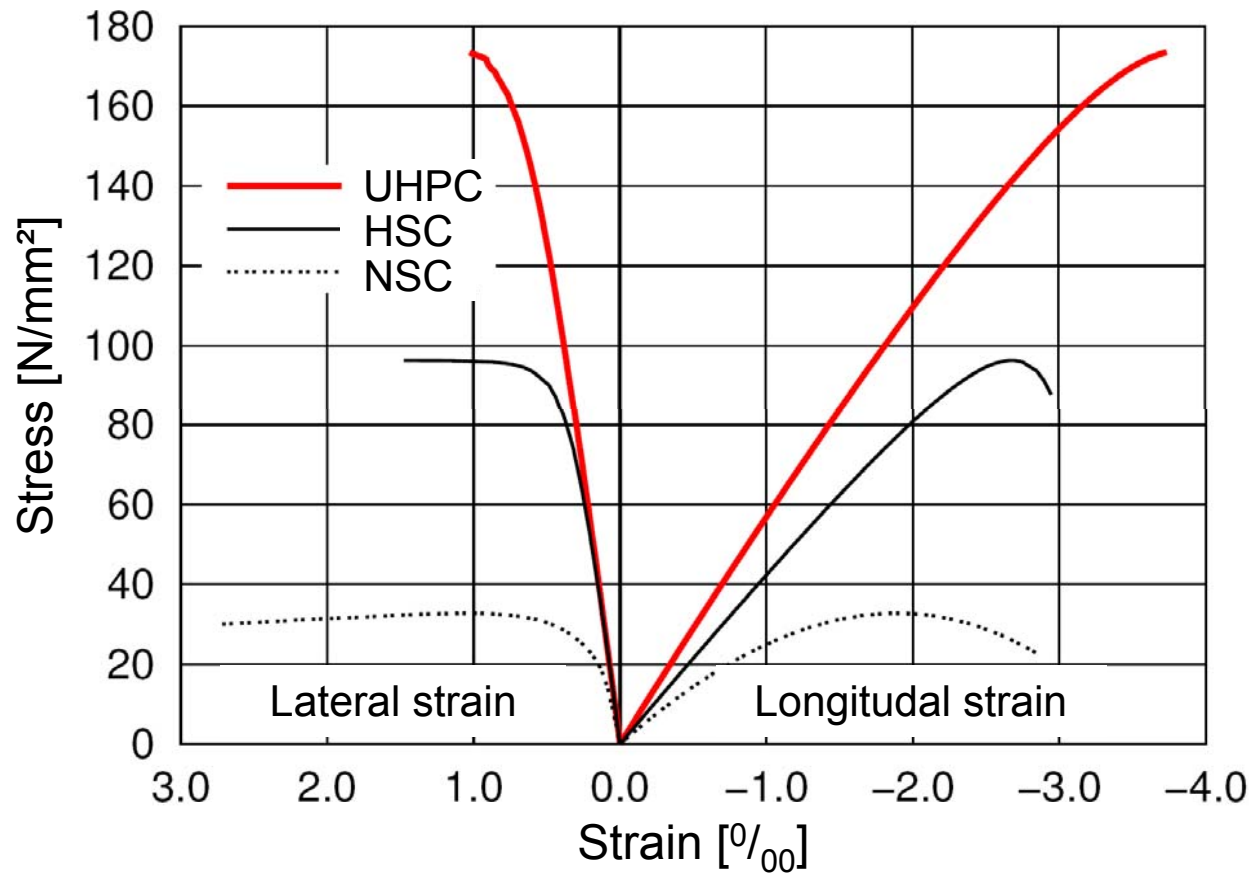
Properties of Fresh UHPC



Flow ability of UHPC

- Composition of raw materials
- Mixing procedure
- Content and type of superplasticizer

Properties of UHPC in Uniaxial Compression



UHPC
172 N/mm²



HSC
96 N/mm²



NSC
36 N/mm²

- High compressive strength ($f_c > 150 \text{ N/mm}^2$)
- High E-Modulus ($E_c > 45000 \text{ N/mm}^2$)
- Linear relationship up to 70 - 80% of the peak

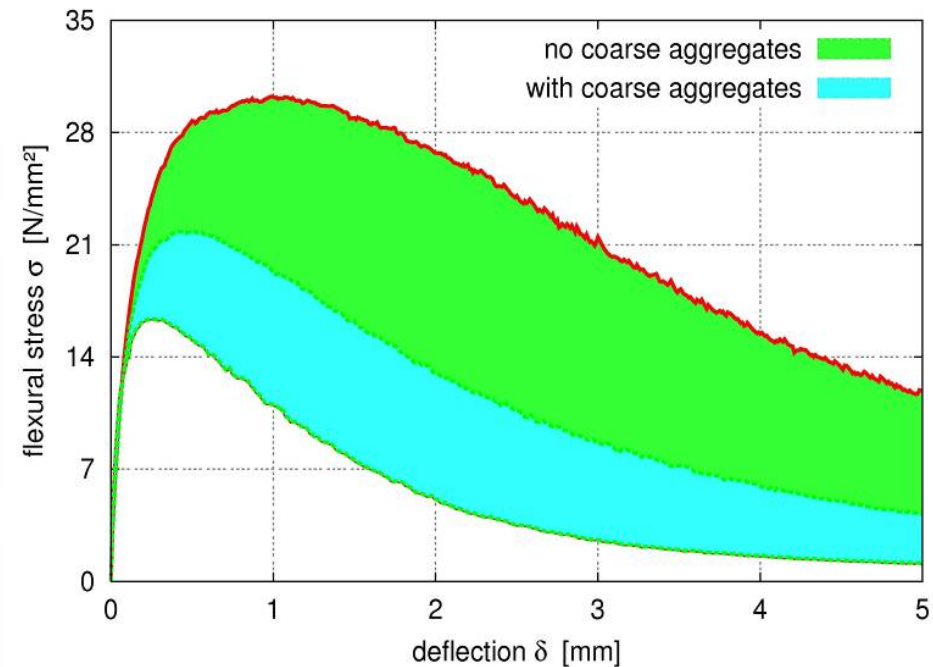
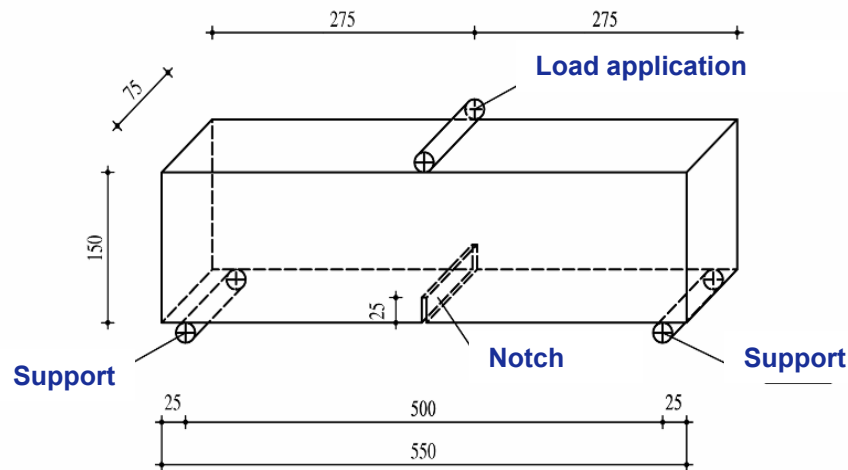
Hardened UHPC in Uniaxial Compression



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Properties of UHPC subjected to Tension

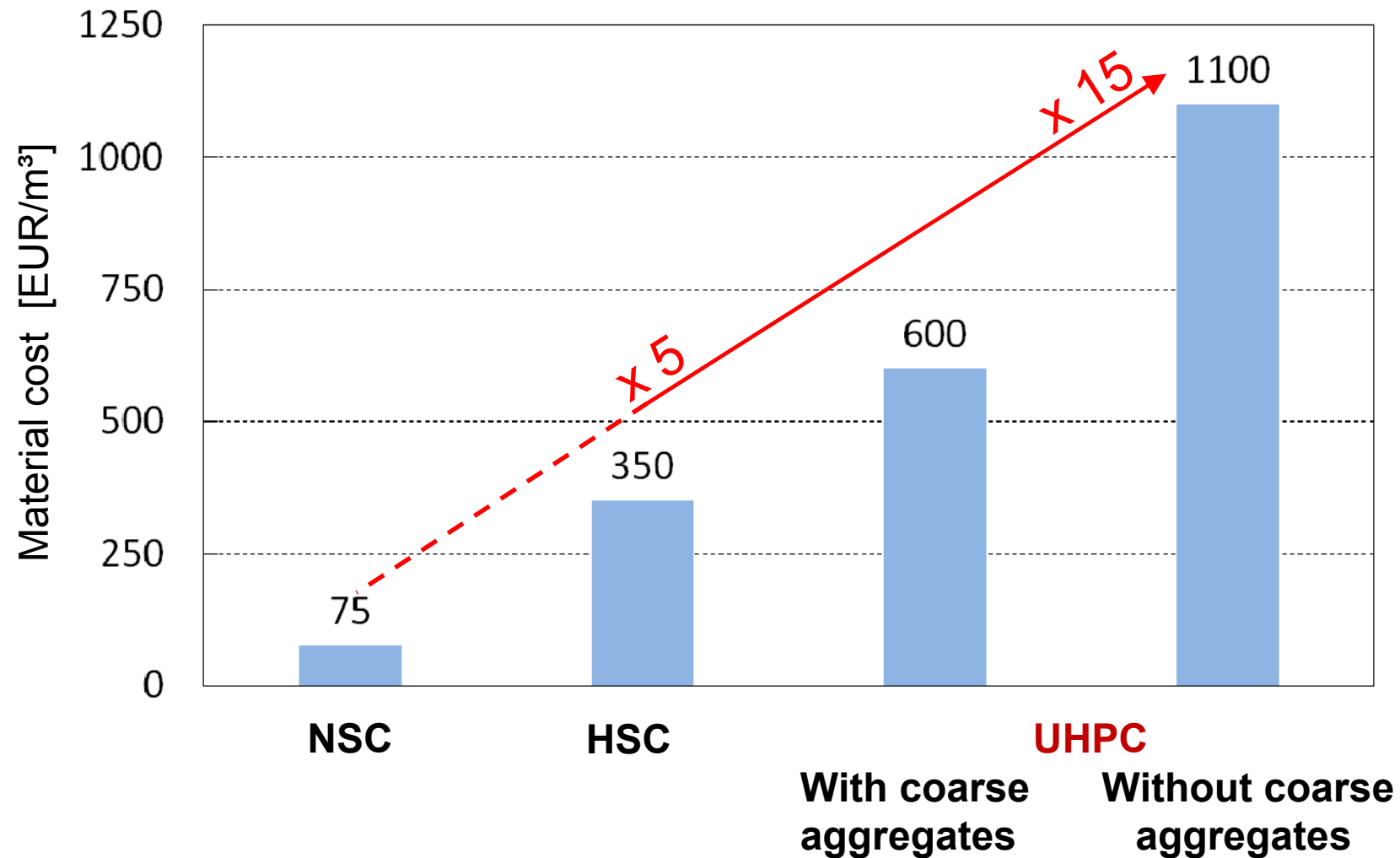
RILEM TC 162-TDF 3-point-bending-test



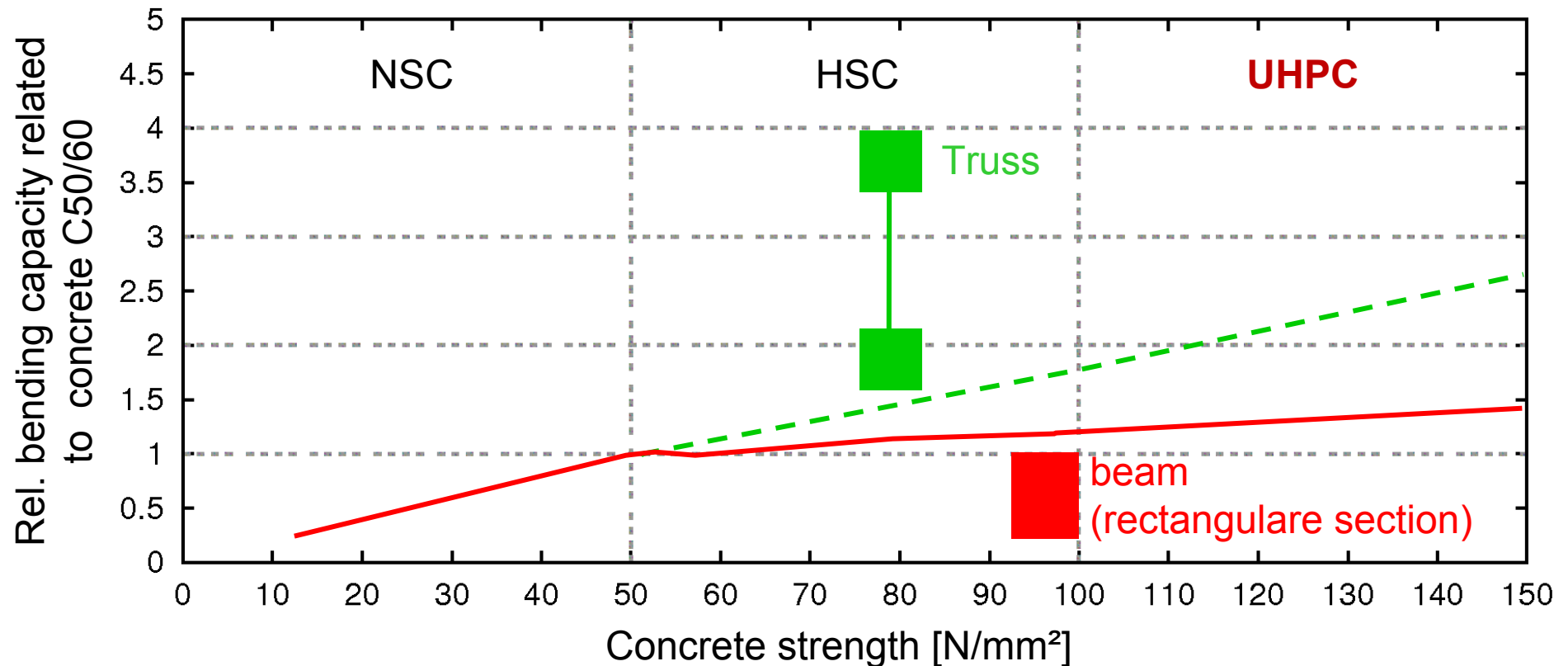
- Fairly ductile behavior in post-peak range
- High flexural tensile strength
- Possible strain hardening due to multiple cracking

⇒ The properties can be optimized though varying fibre geometry & contents!

A Comparison of the Material Costs



Comparison of Bending Capacity of NSC and HSC



⇒ In conventional concrete structures, the advantages
of UHPC can not be fully used!!!

Properties of Fresh UHPC

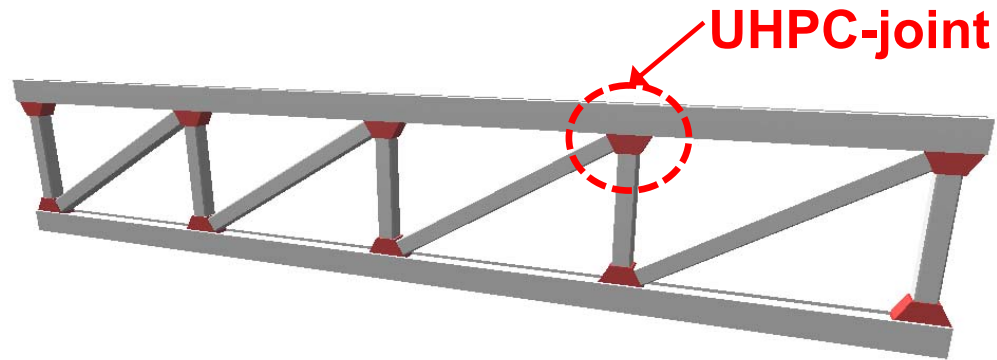
„Elephant skin“ on the casting surface
(about 15 minutes after mixing)



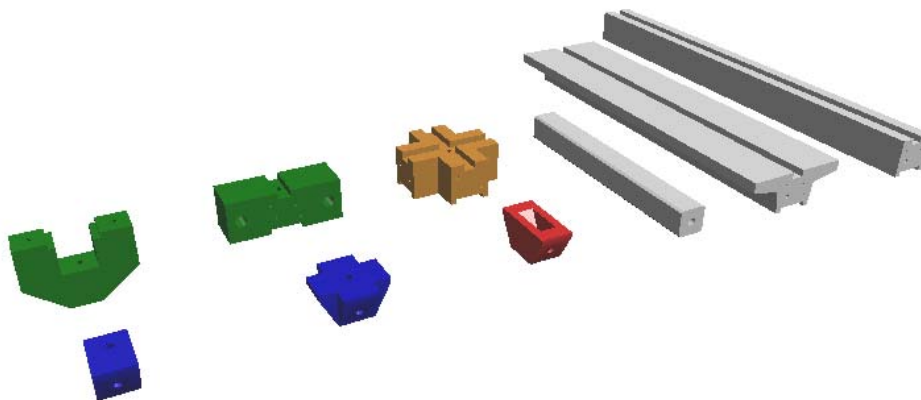
⇒ The using UHPC in precast elements is recommended!

Modular Truss System made of UHPC

Side View



Elements

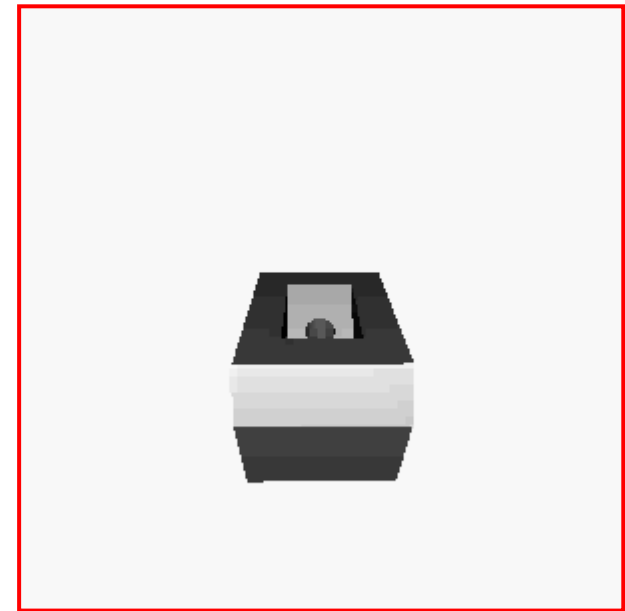
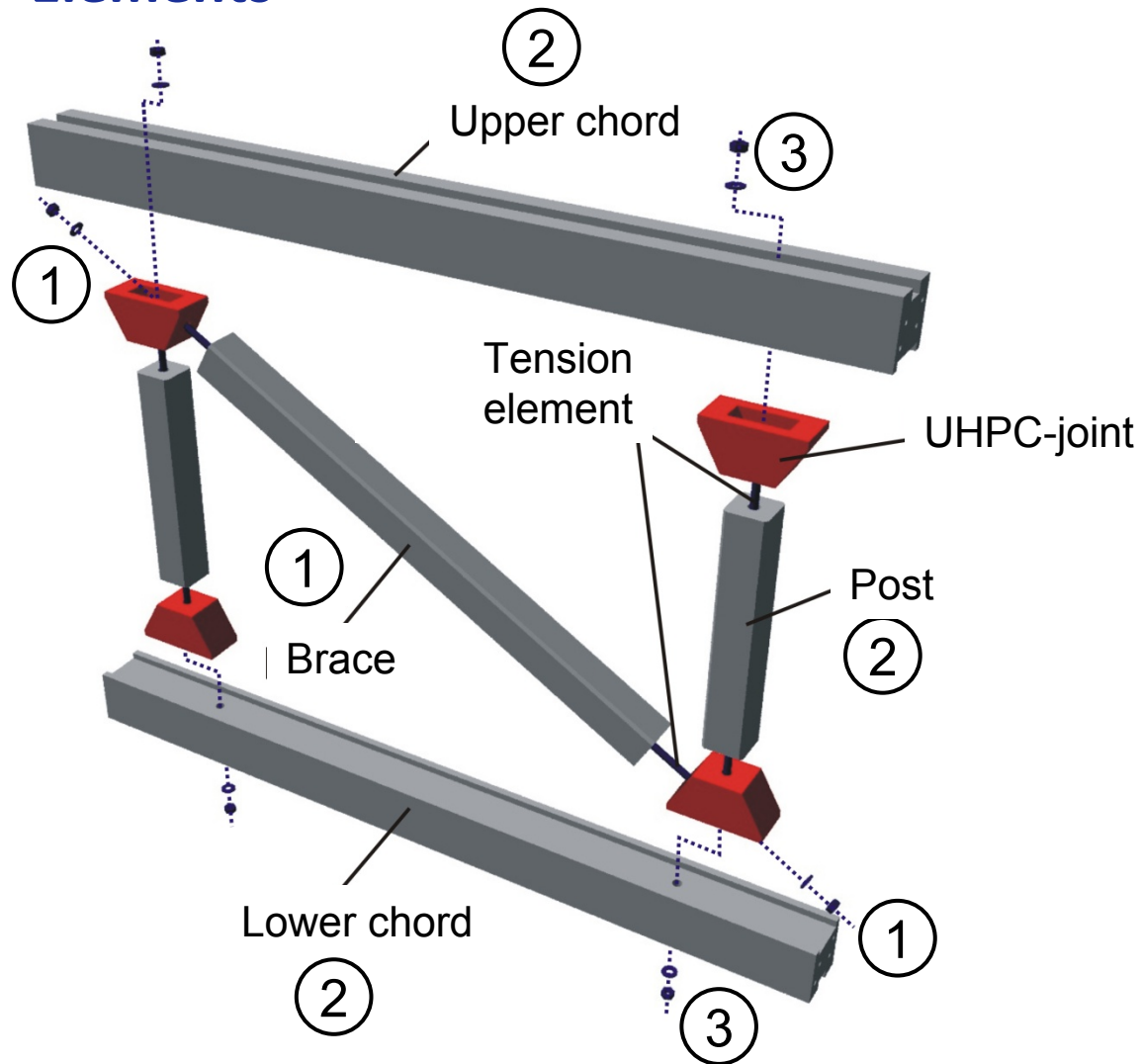


Advantages

- No monolithic joint
- Dependence of the truss only on the joint element
- Serge's fabrication of the truss member
- Assembling in precast plant and on site possible
- Non-destructive disassembling and reutilization of the construction

Modular Truss System made of UHPC

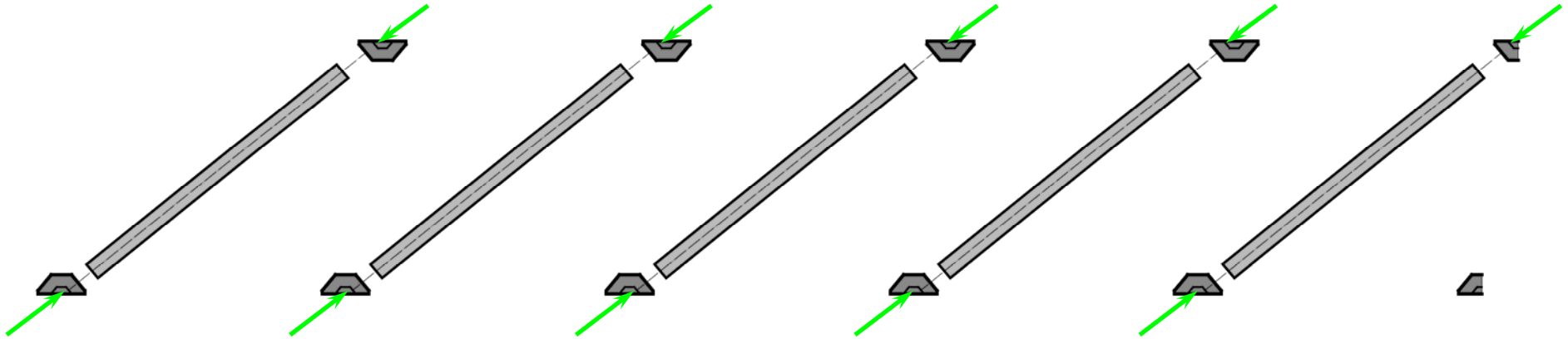
Elements



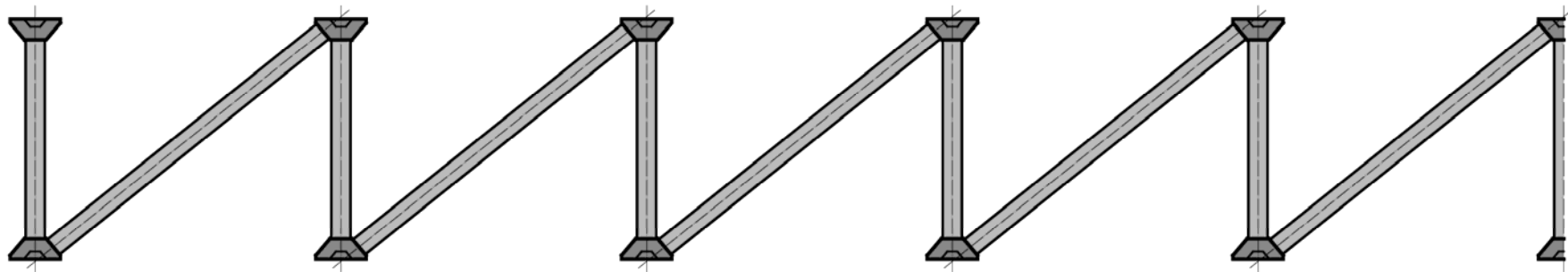
UHPC-joint

Modular Truss System made of UHPC

Assembling of the joints / posts

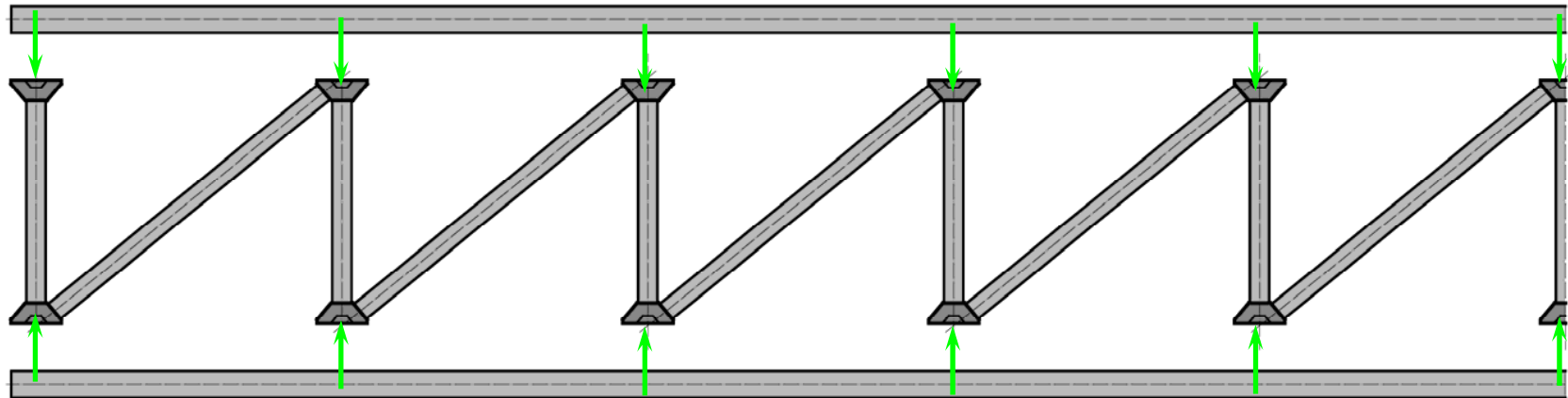


Assembling of the braces

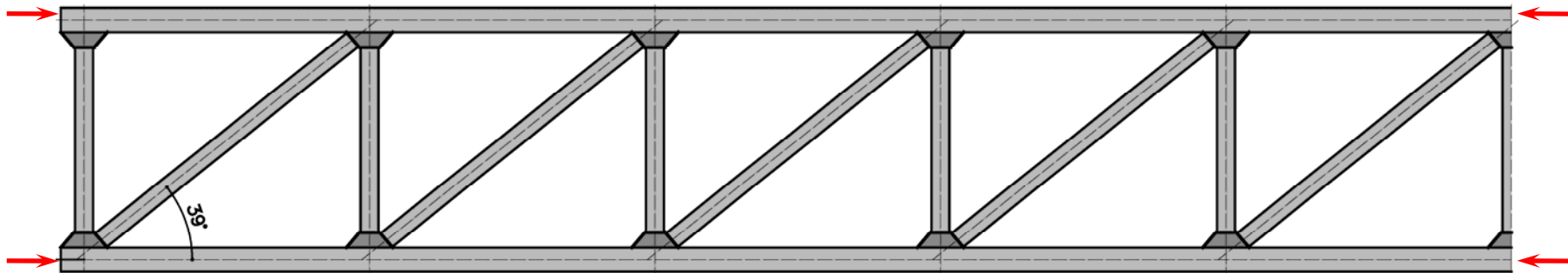


Modular Truss System made of UHPC

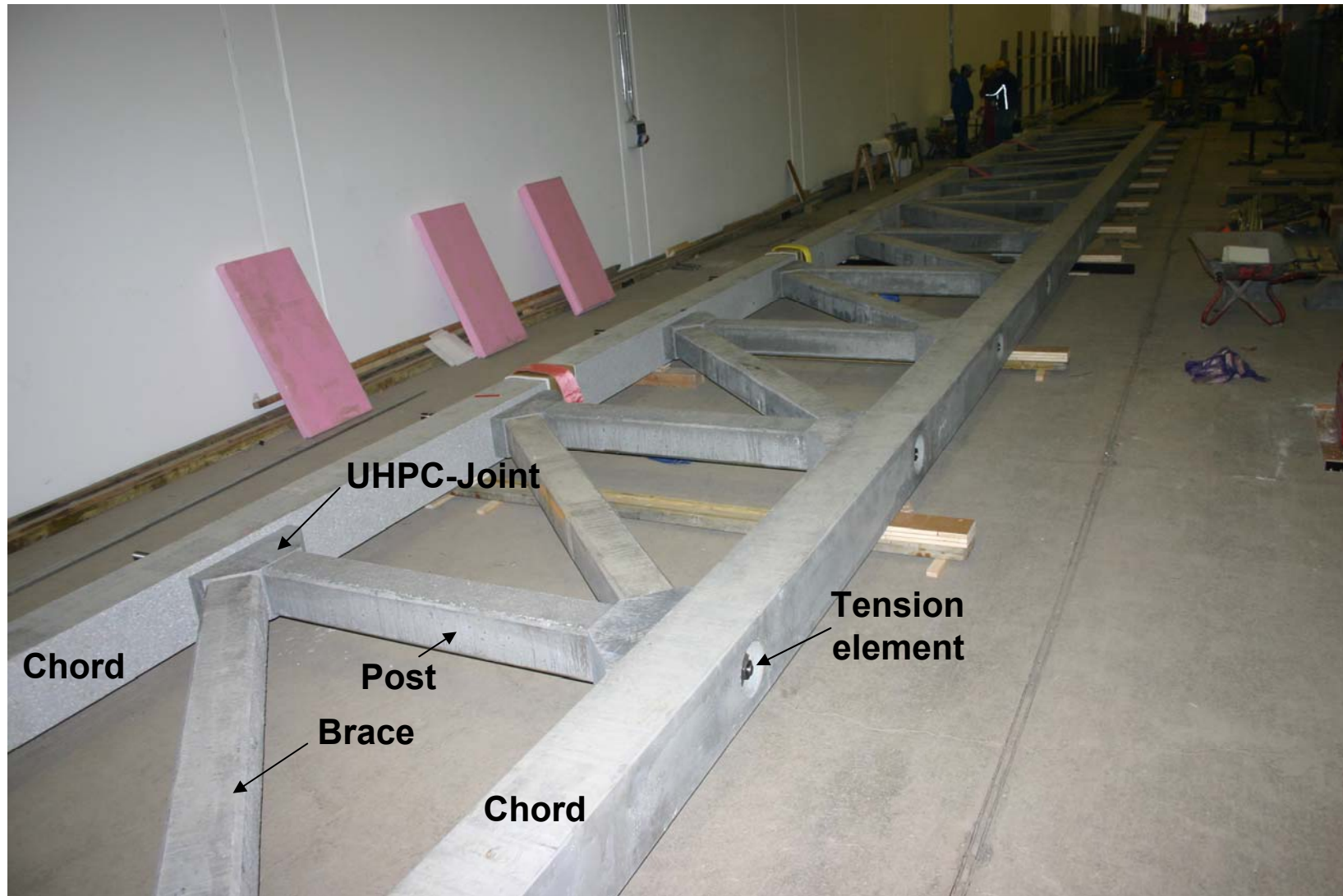
Montage of the joints / posts



Prestressing of the chords

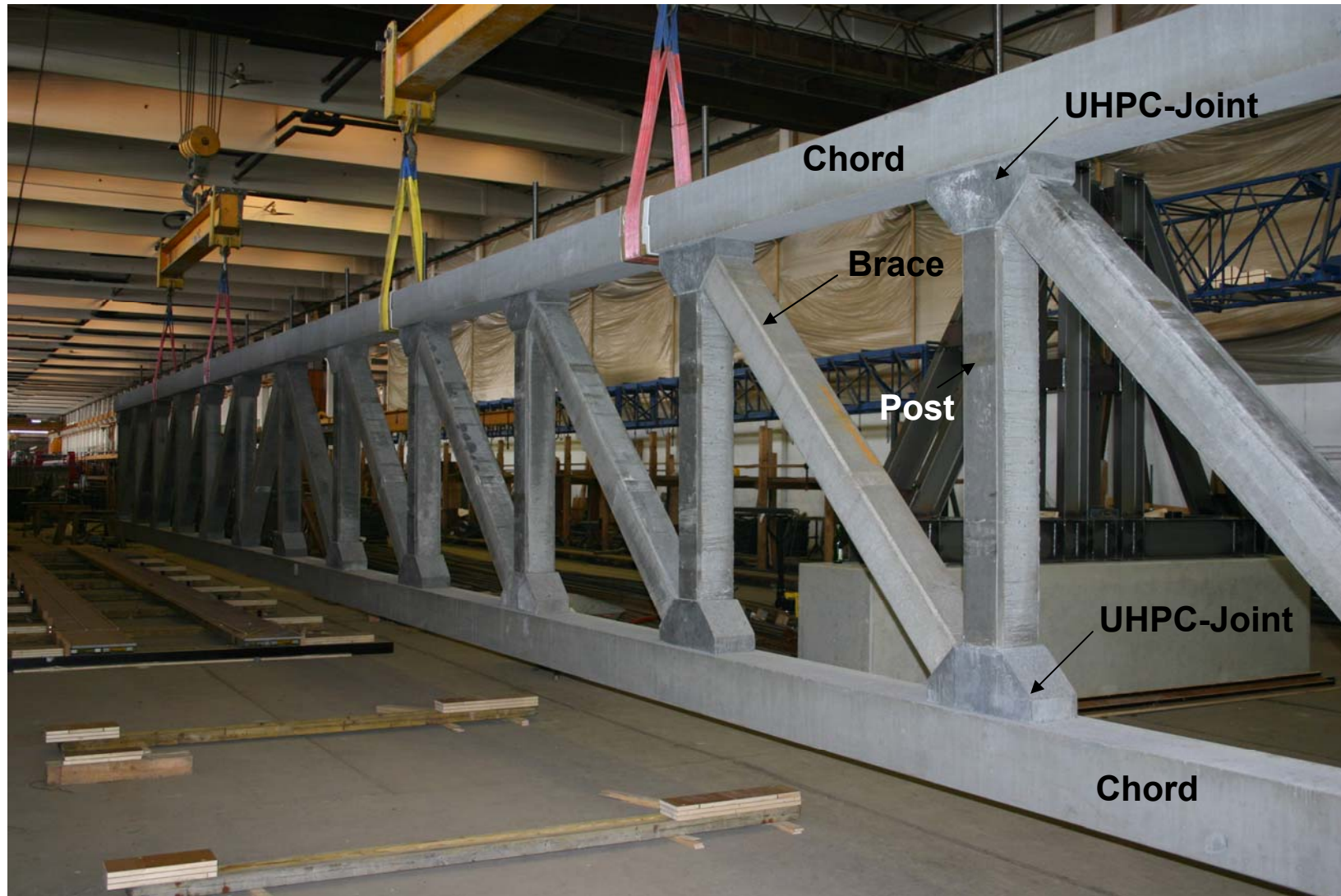


Modular Truss System made of UHPC



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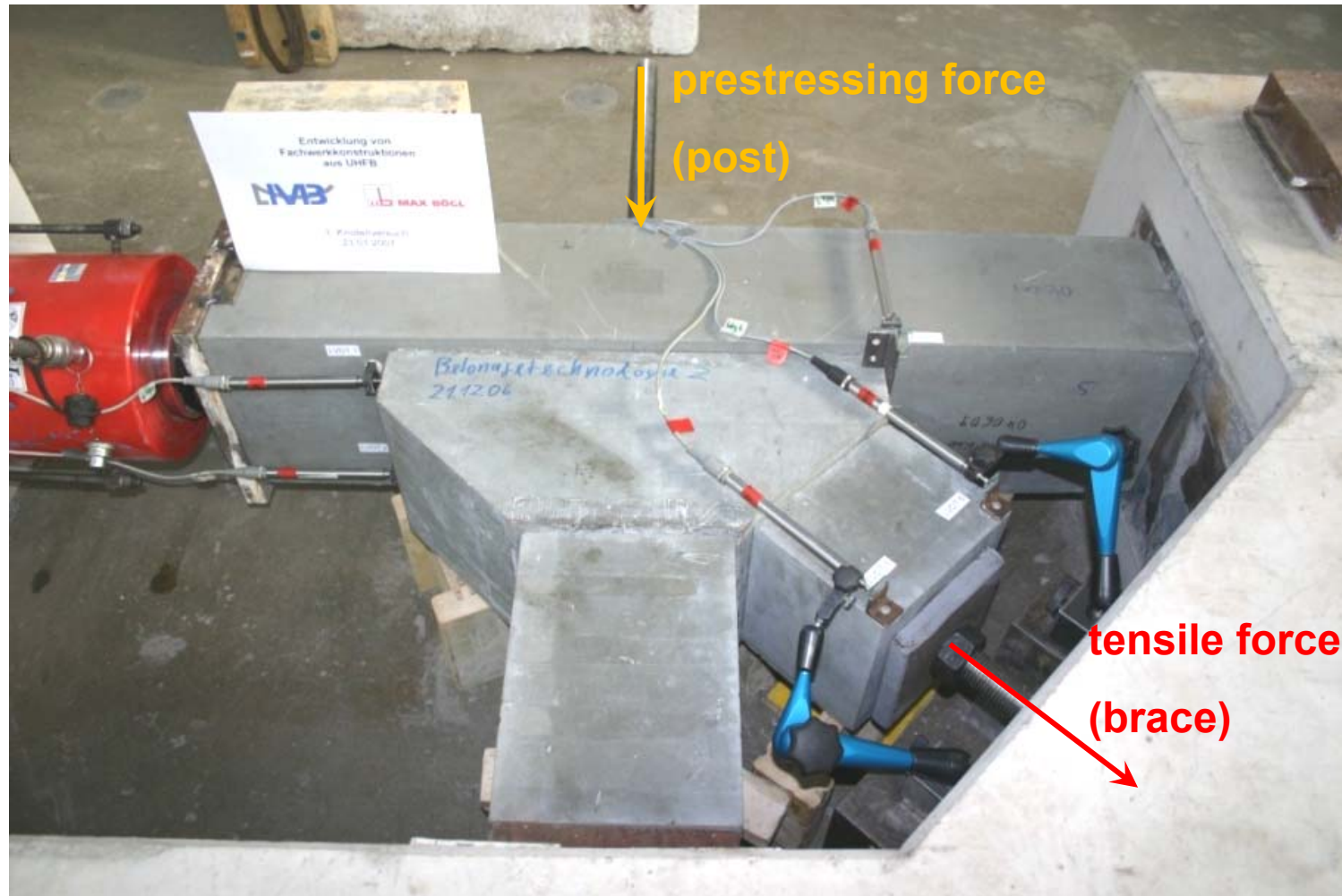
Modular Truss System made of UHPC



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Modular Truss System made of UHPC

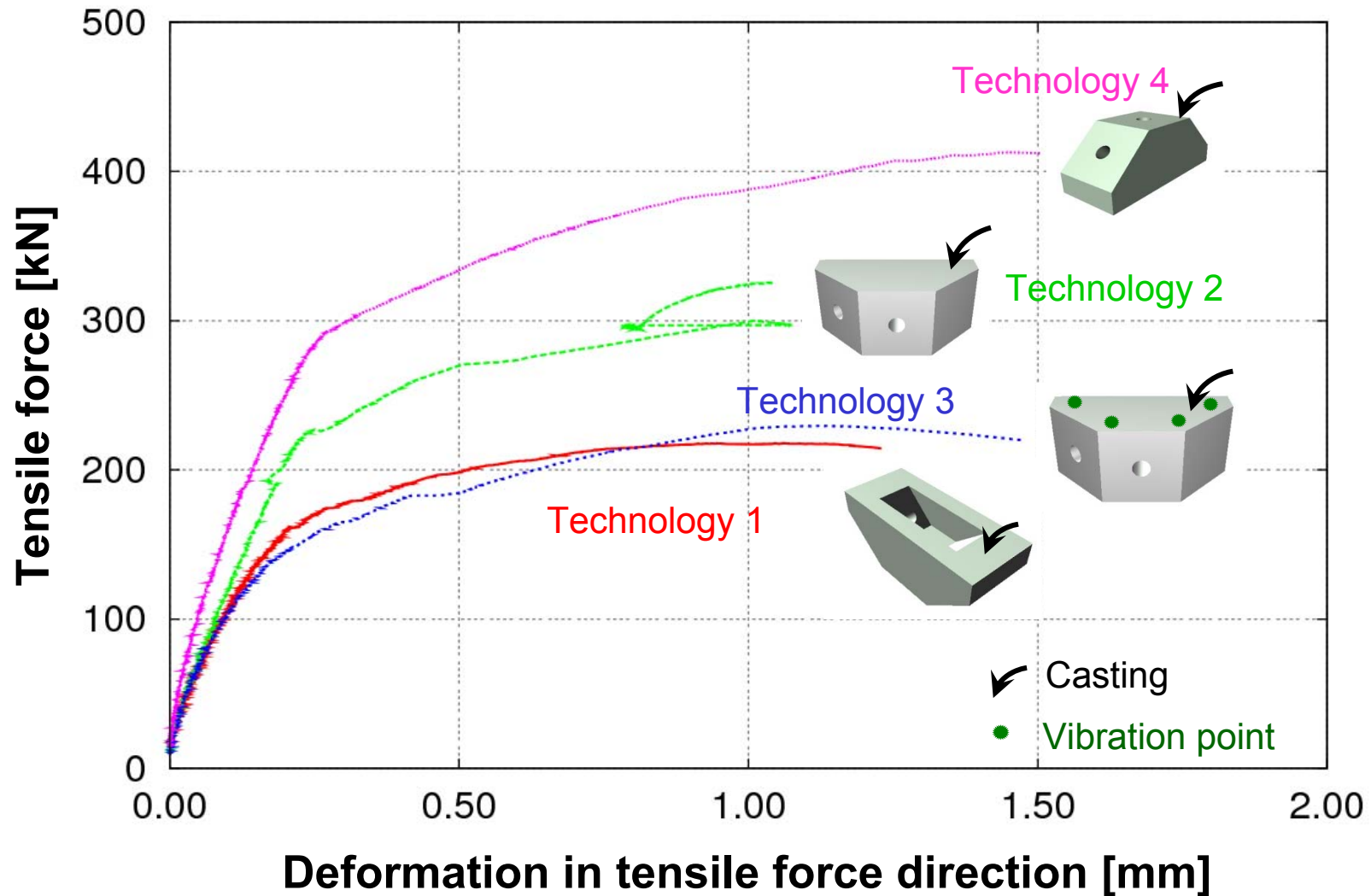
Test Set-up for UHPC-joint



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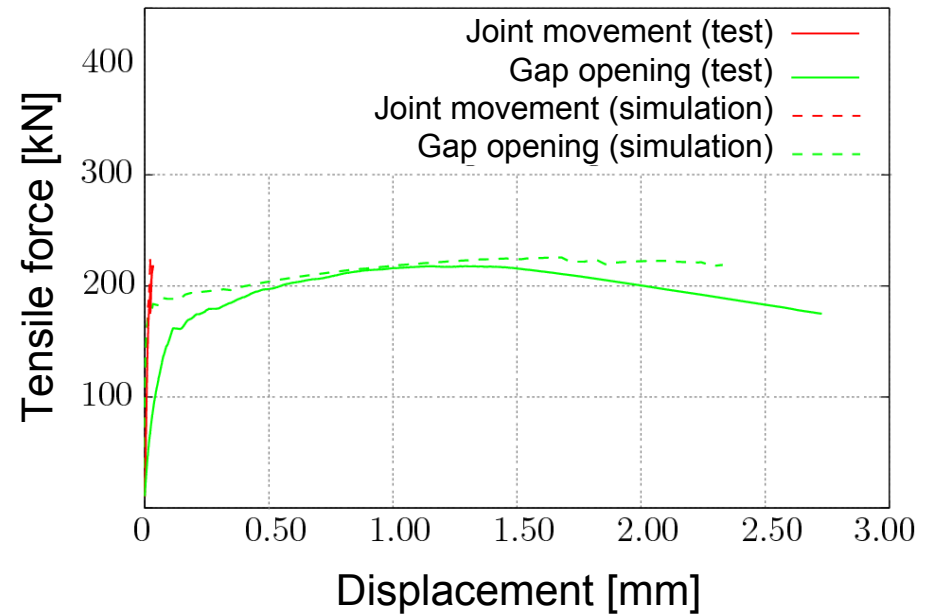
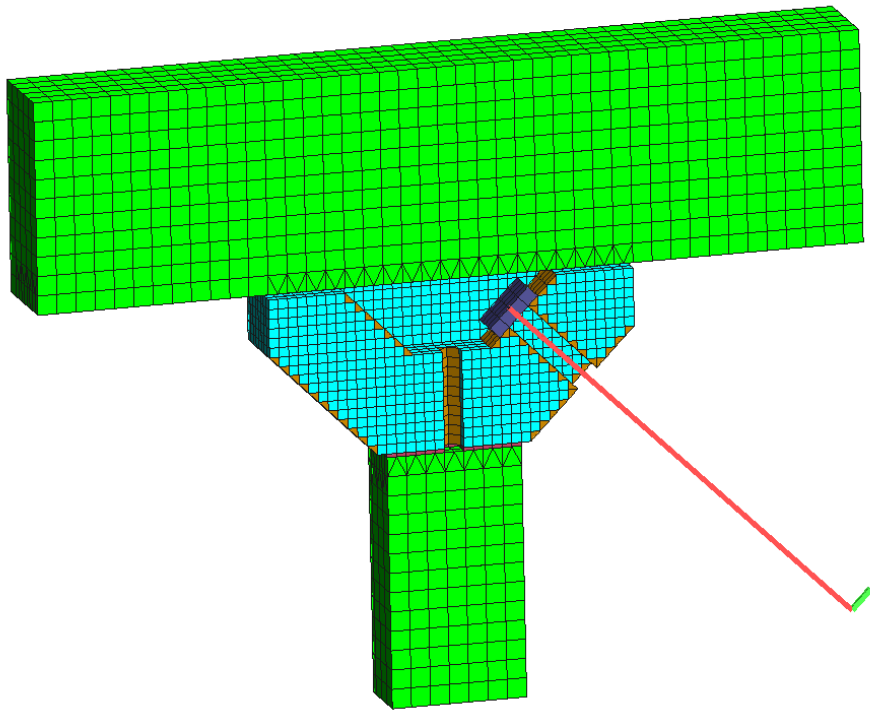
Modular Truss System made of UHPC

Influence of the Casting Technology

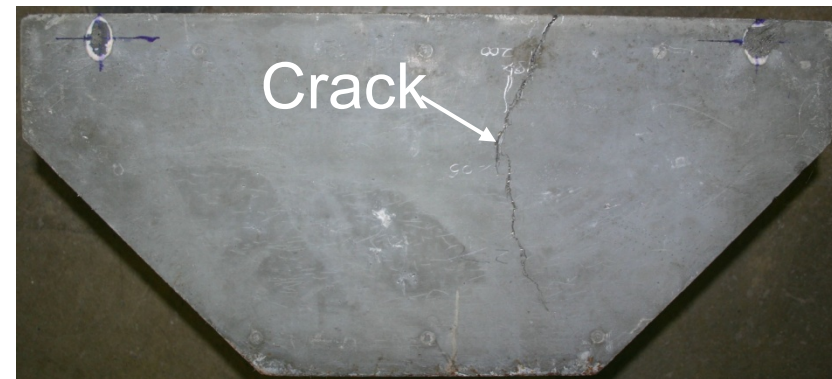


Modular Truss System made of UHPC

Numerical Simulations

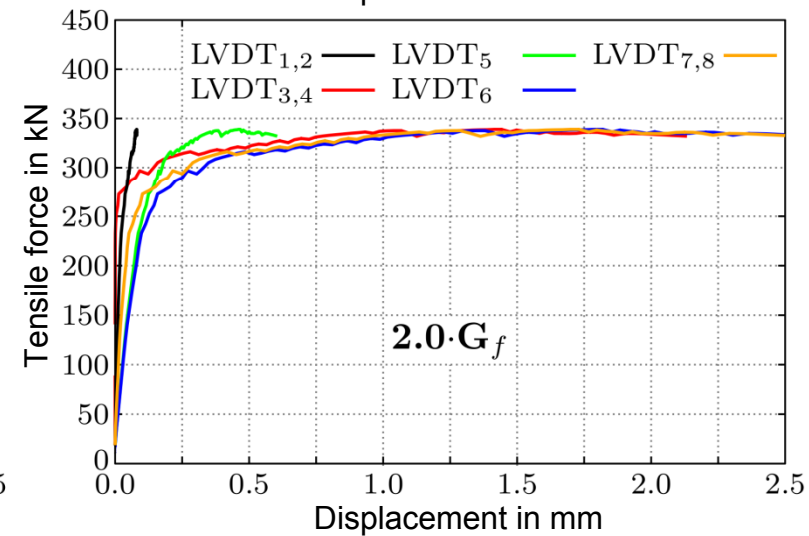
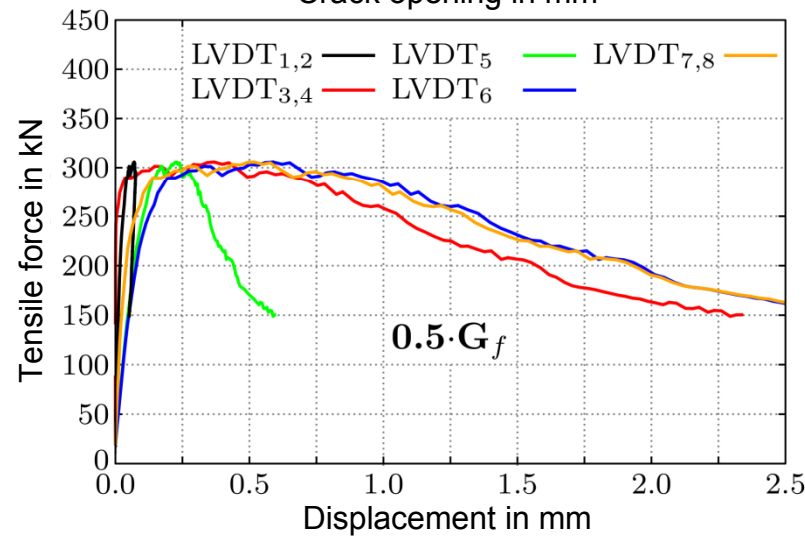
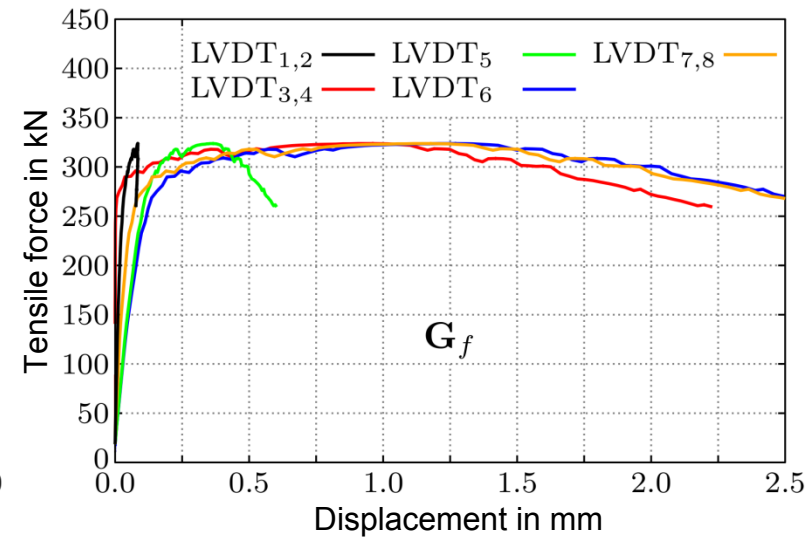
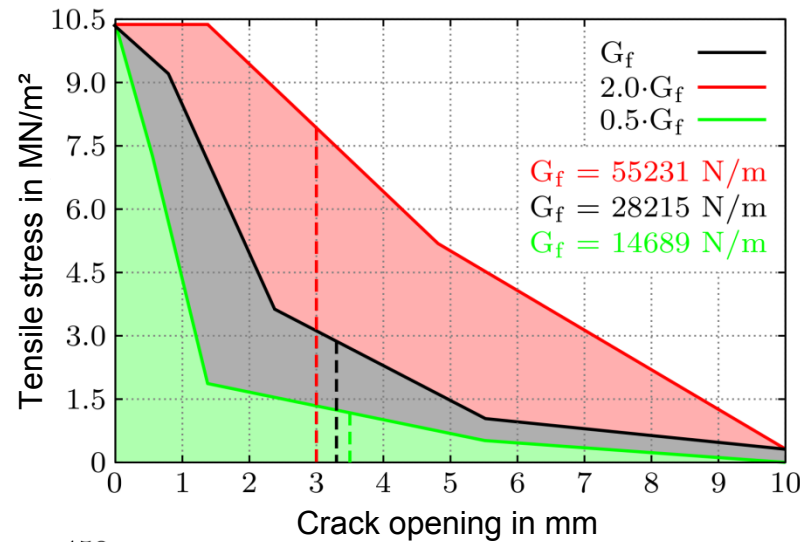


- ATENA software package
- Fracture-plastic model for concrete
- Interface element

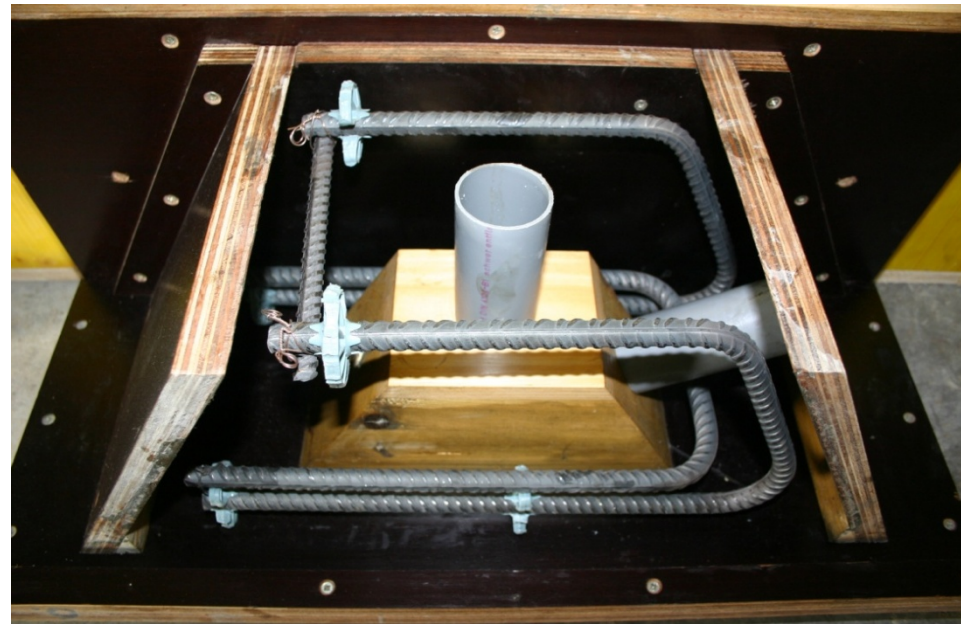
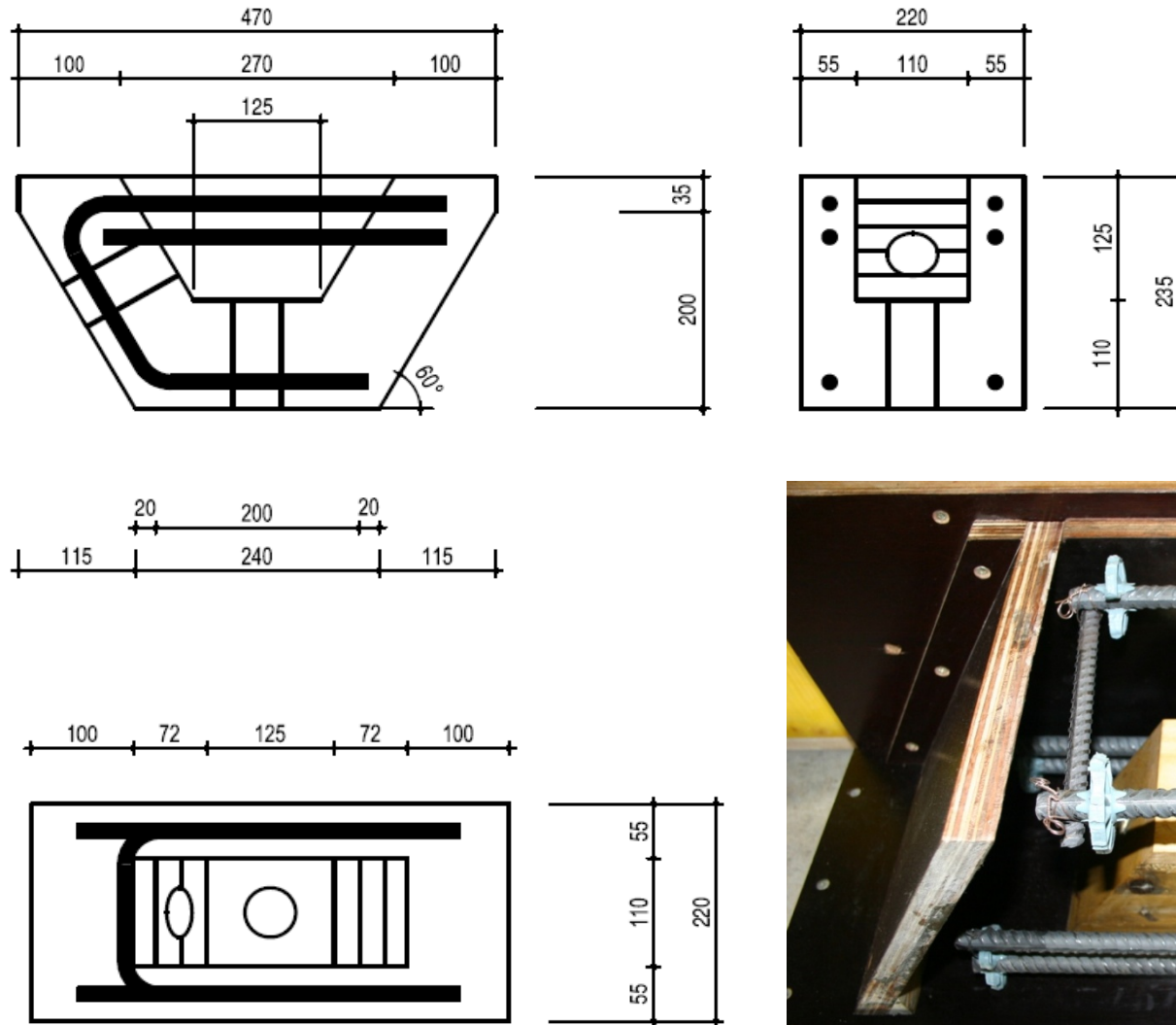


Modular Truss System made of UHPC

Numerical Simulations

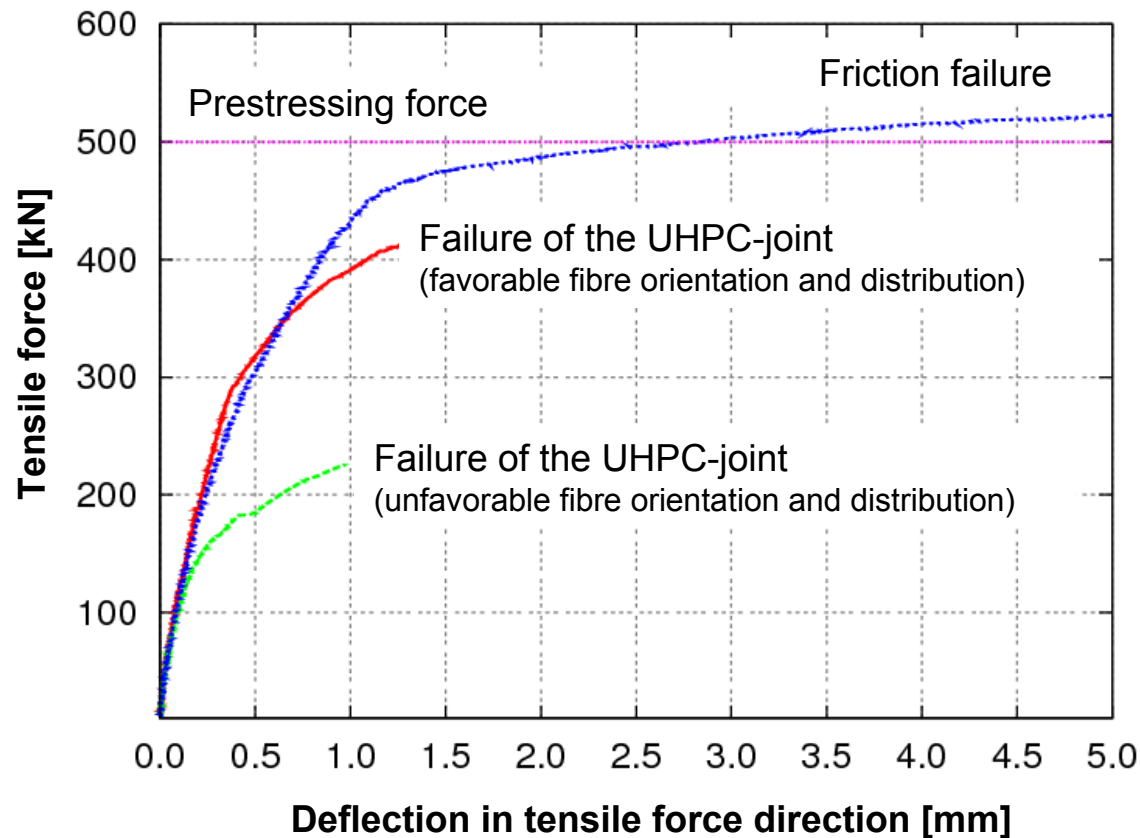


Modular Truss System made of UHPC



Modular Truss System made of UHPC

UHPC-joint with Brace Inclination of 45°



a) **without** reinforcement:

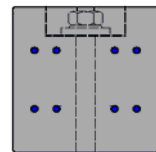
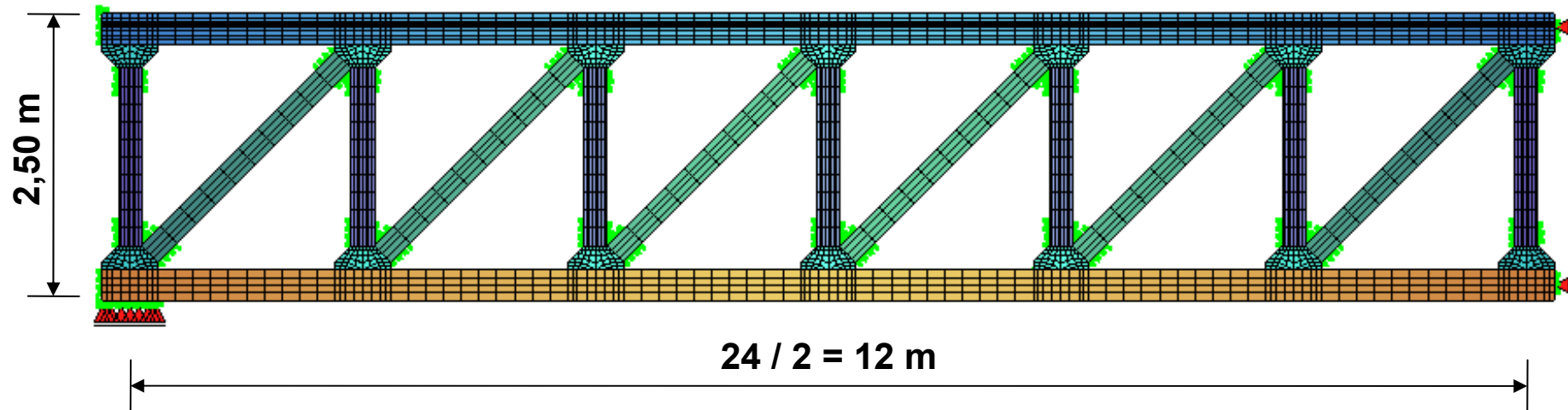
- Brittle failure of the UHPC-joint
- Load capacity is influenced by the fibre orientation and fibre distribution

b) **With** reinforcement:

- Ductile failure of the friction between UHPC-joint and chord ($\mu \approx 0.65$)
- Friction can be increased up to $\mu \approx 0.80$

Modular Truss System made of UHPC

Influence of the Production Tolerance



Chord
 $b/h = 30/30$ cm



Brace/ post
 $b/h = 30/30$ cm

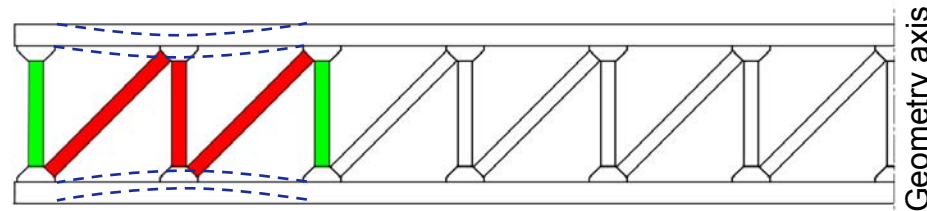
Prestressing force: $P = 950$ kN (upper chord)
 $P = 950$ kN (lower chord)

$P = 100$ kN (brace)
 $P = 660$ kN (post)

Modular Truss System made of UHPC

Influence of the Production Tolerance

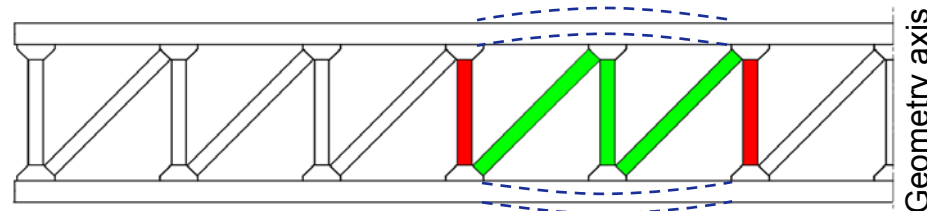
- Upper chord



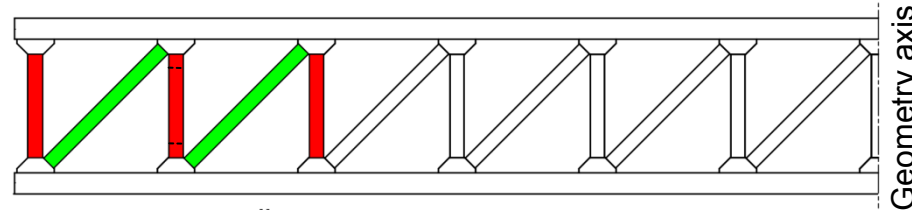
■ Short element

■ Long element

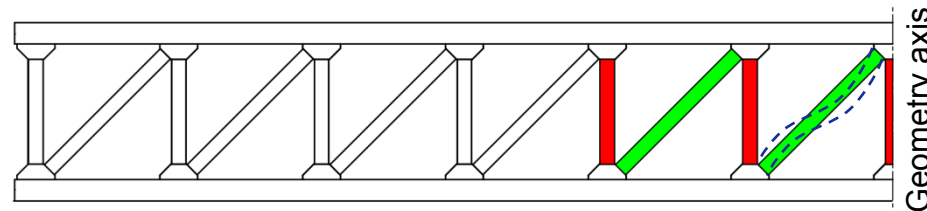
- Lower chord



- Post



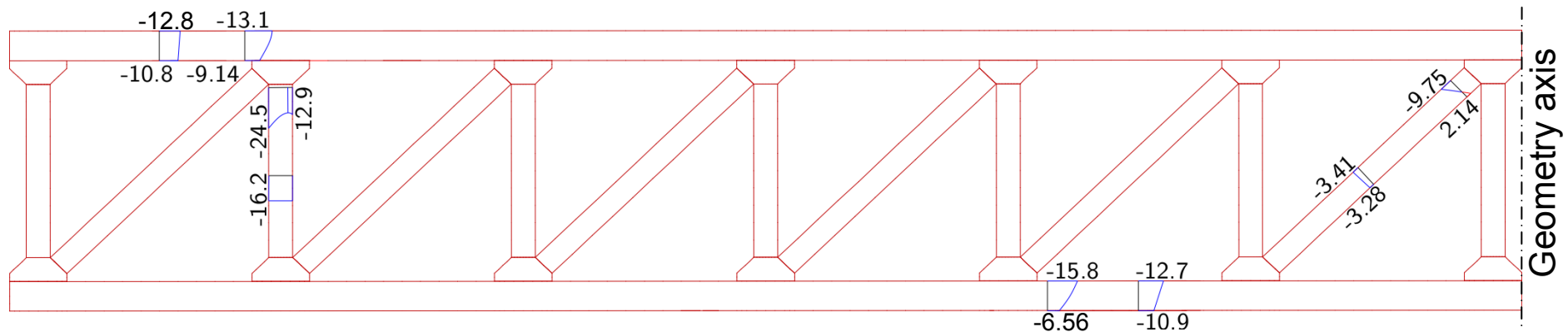
- Brace



Modular Truss System made of UHPC

Influence of the Production Tolerance

Production tolerance $\Delta L = \pm 0,50 \text{ mm}$



Tensile stress due to production tolerance

Upper chord	Lower chord	Brace	Post
+ 3,16 MN/m ²	+ 3,20 MN/m ²	+ 2,19 MN/m ²	+ 2,10 MN/m ²

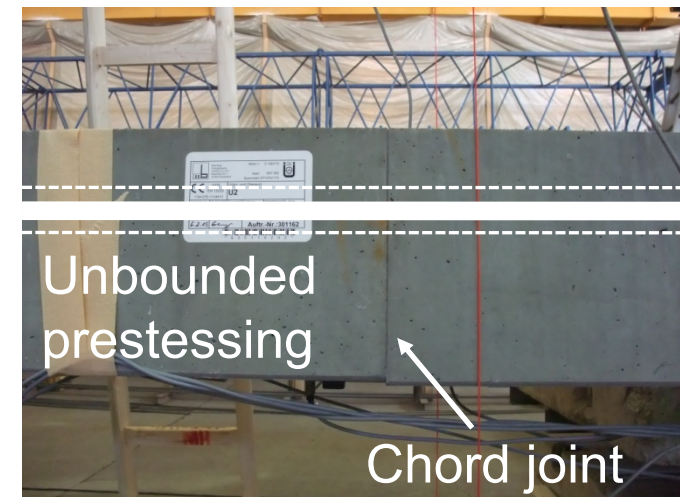
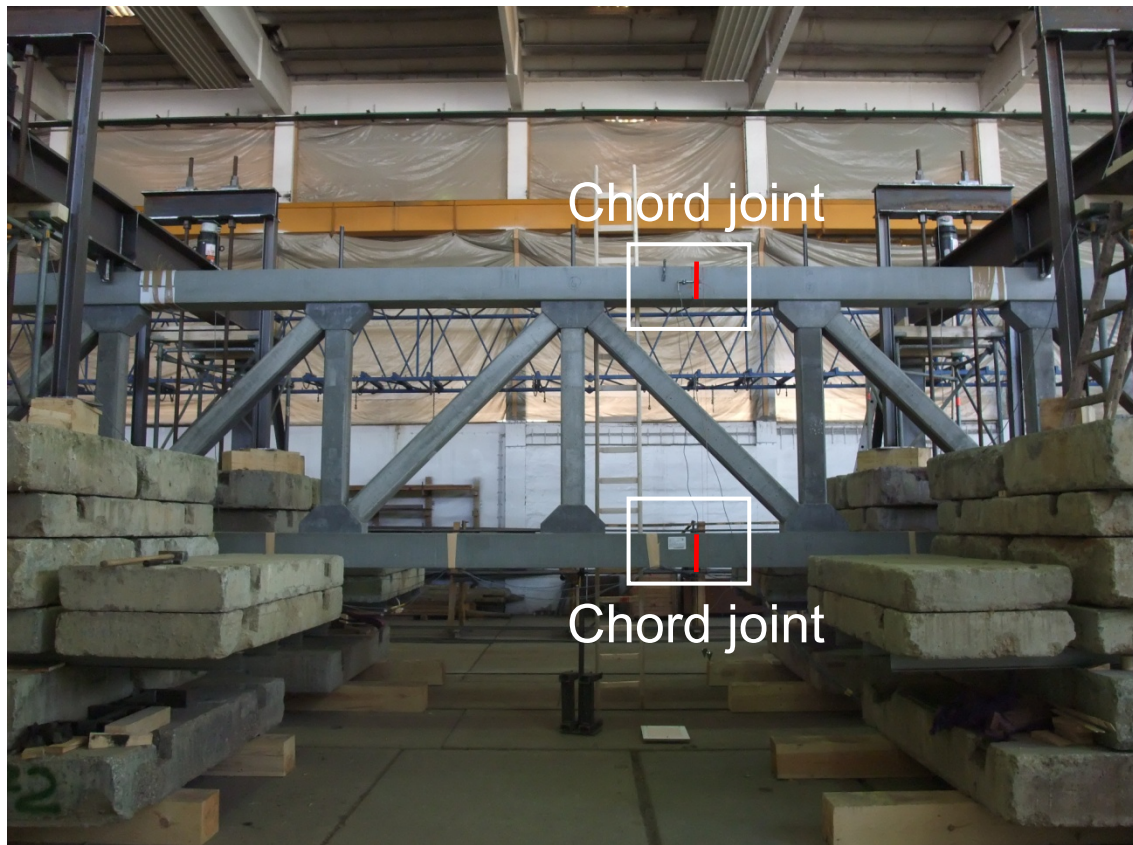
Modular Truss System made of UHPC

Test Set-up for full scale test

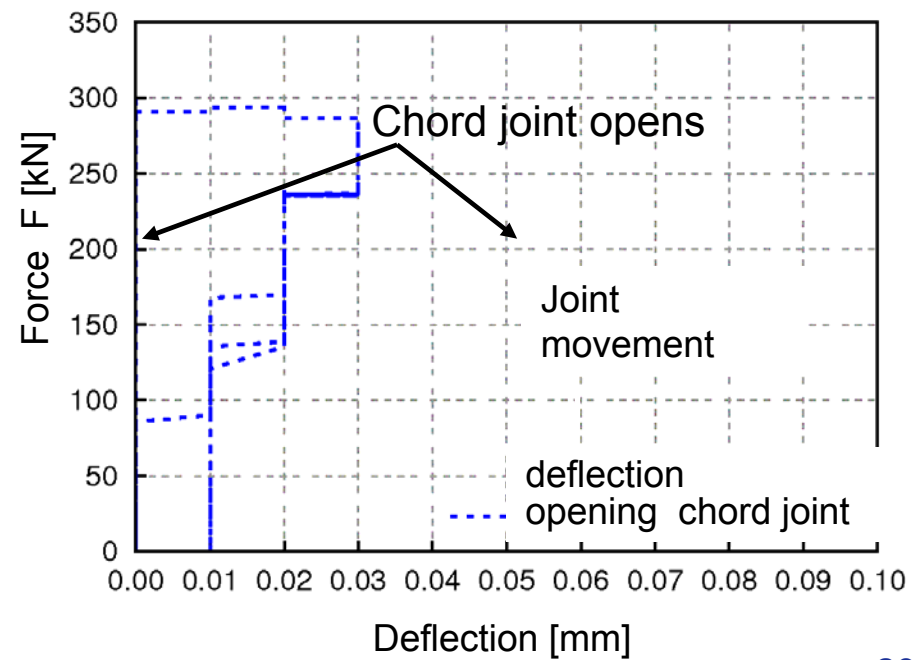
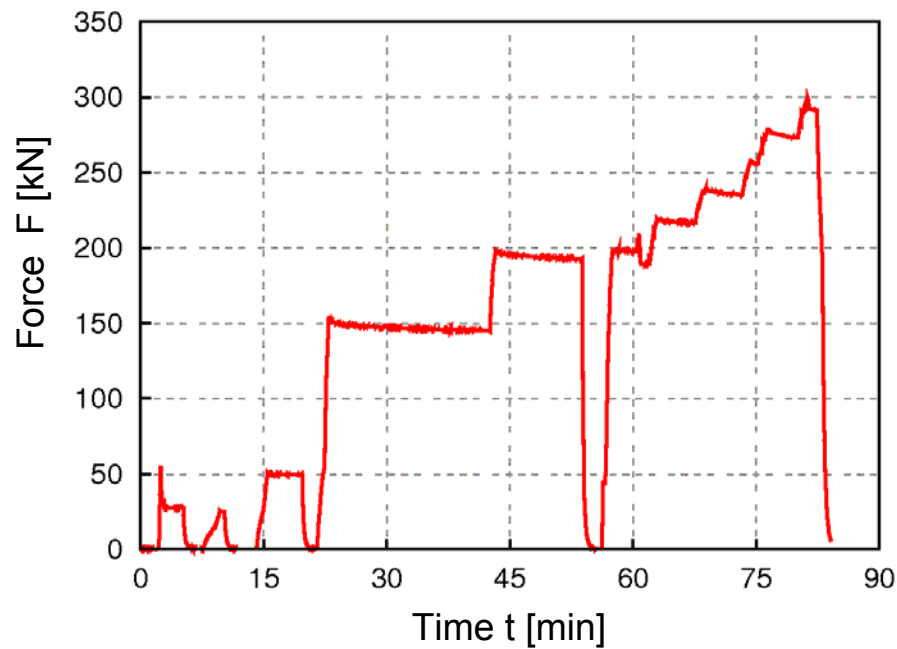
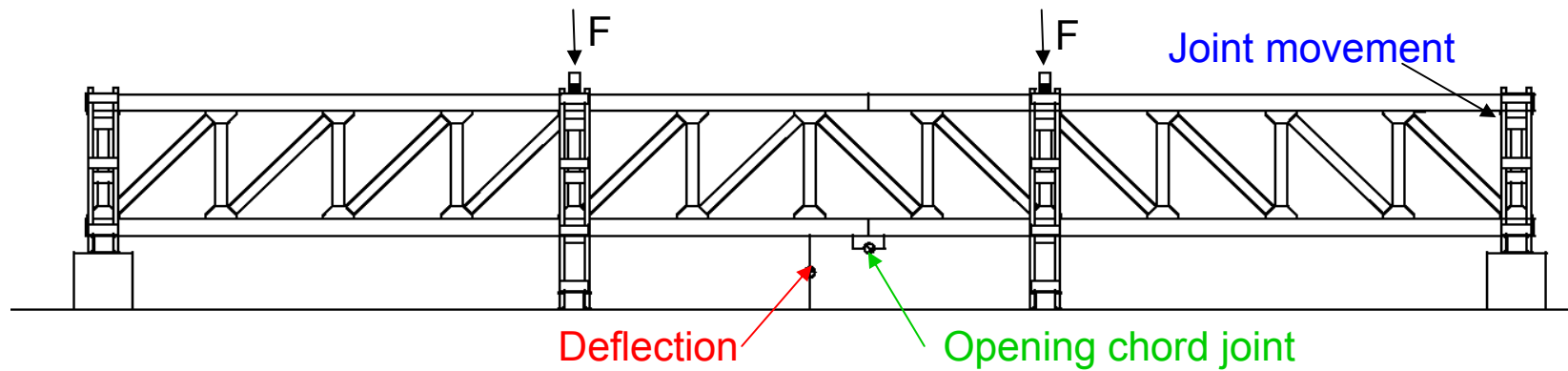


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Modular Truss System made of UHPC



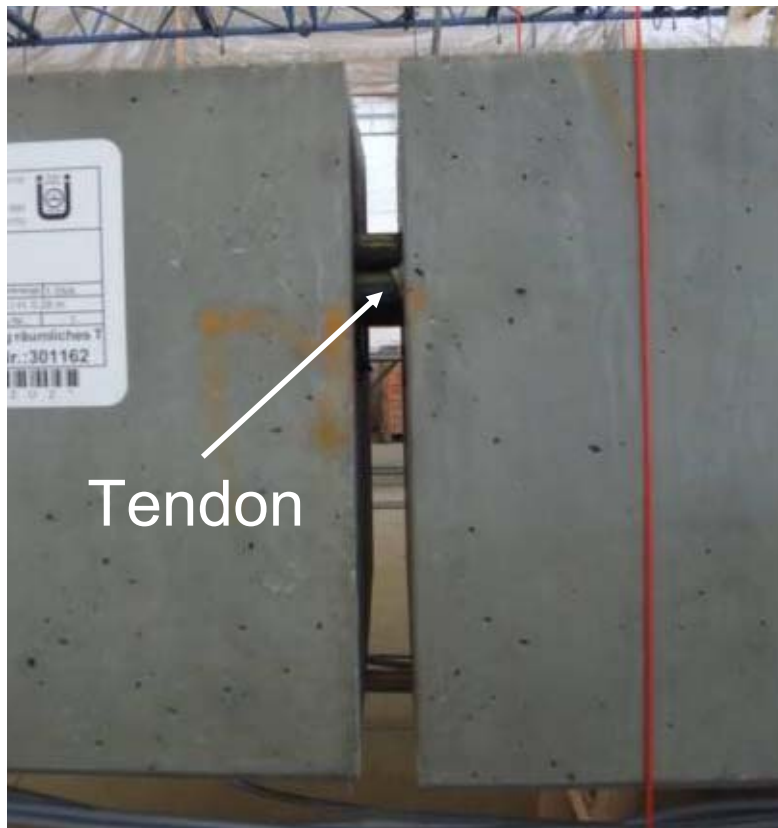
Modular Truss System made of UHPC



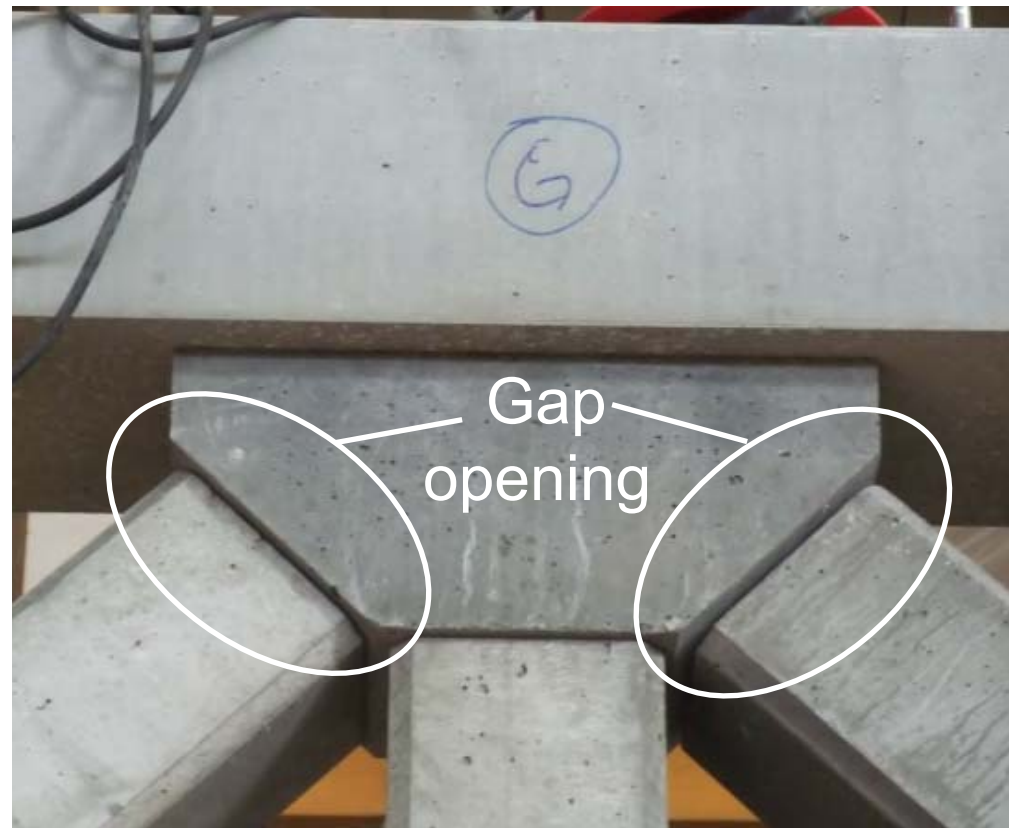
Modular Truss System made of UHPC

Failure of Truss System

lower chord

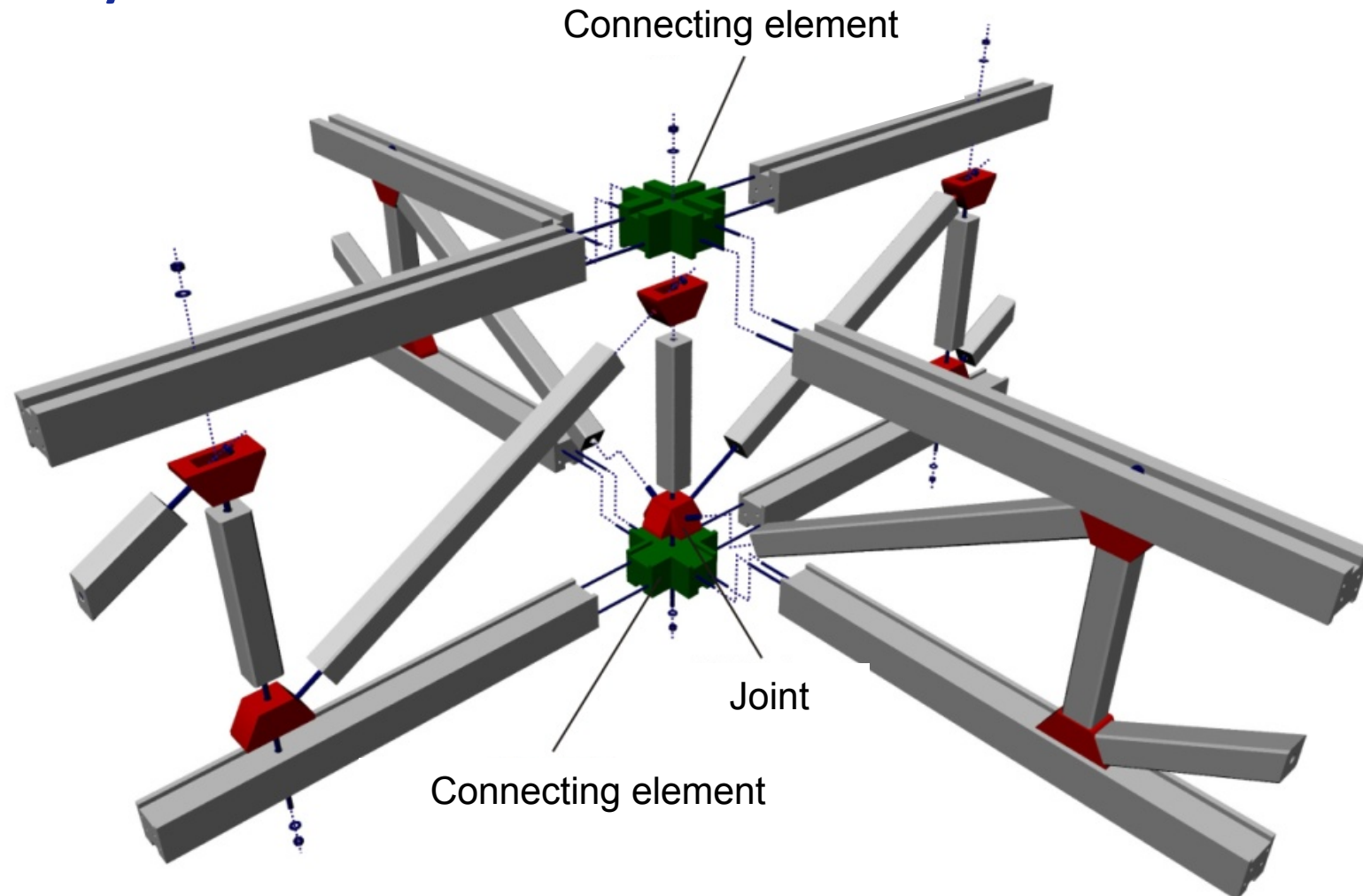


Upper chord



Modular Truss System made of UHPC

space System



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The Gärtnerplatz Bridge, Germany

A Hybrid UHPC-Steel Bridge



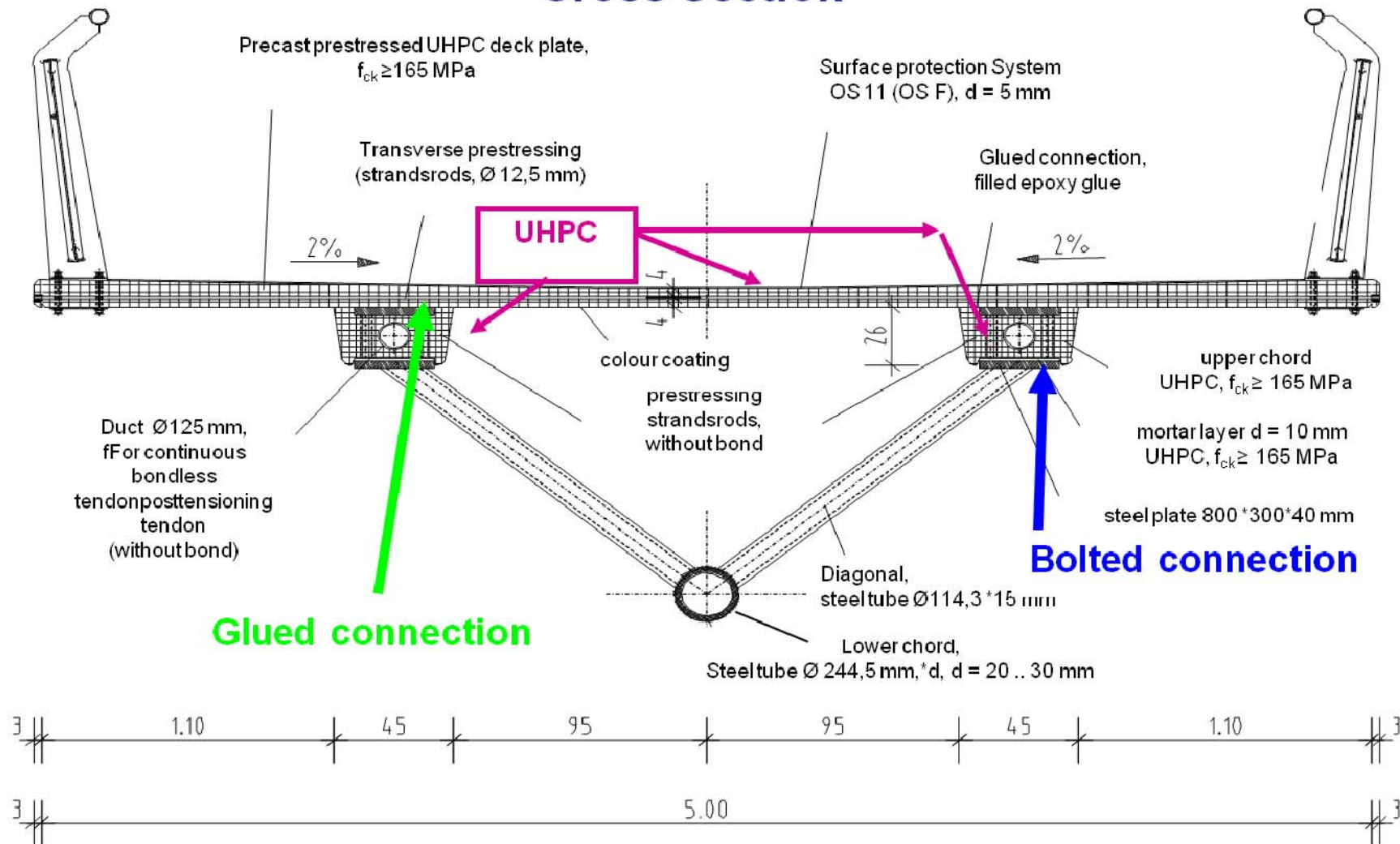
Ekkehard FEHLING, Professor; University of Kassel, Germany

Kai BUNJE, Civil Engineer, IBB Fehling+Jungmann, Kassel, Germany

Michael SCHMIDT, **Walter SCHREIBER**

The Gärtnerplatz Bridge

Cross Section

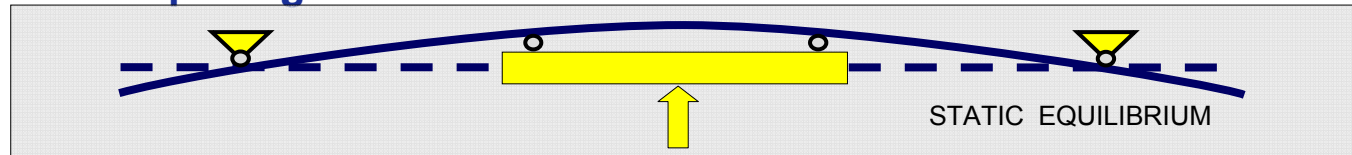


The Gärtnerplatz Bridge

Bending Test

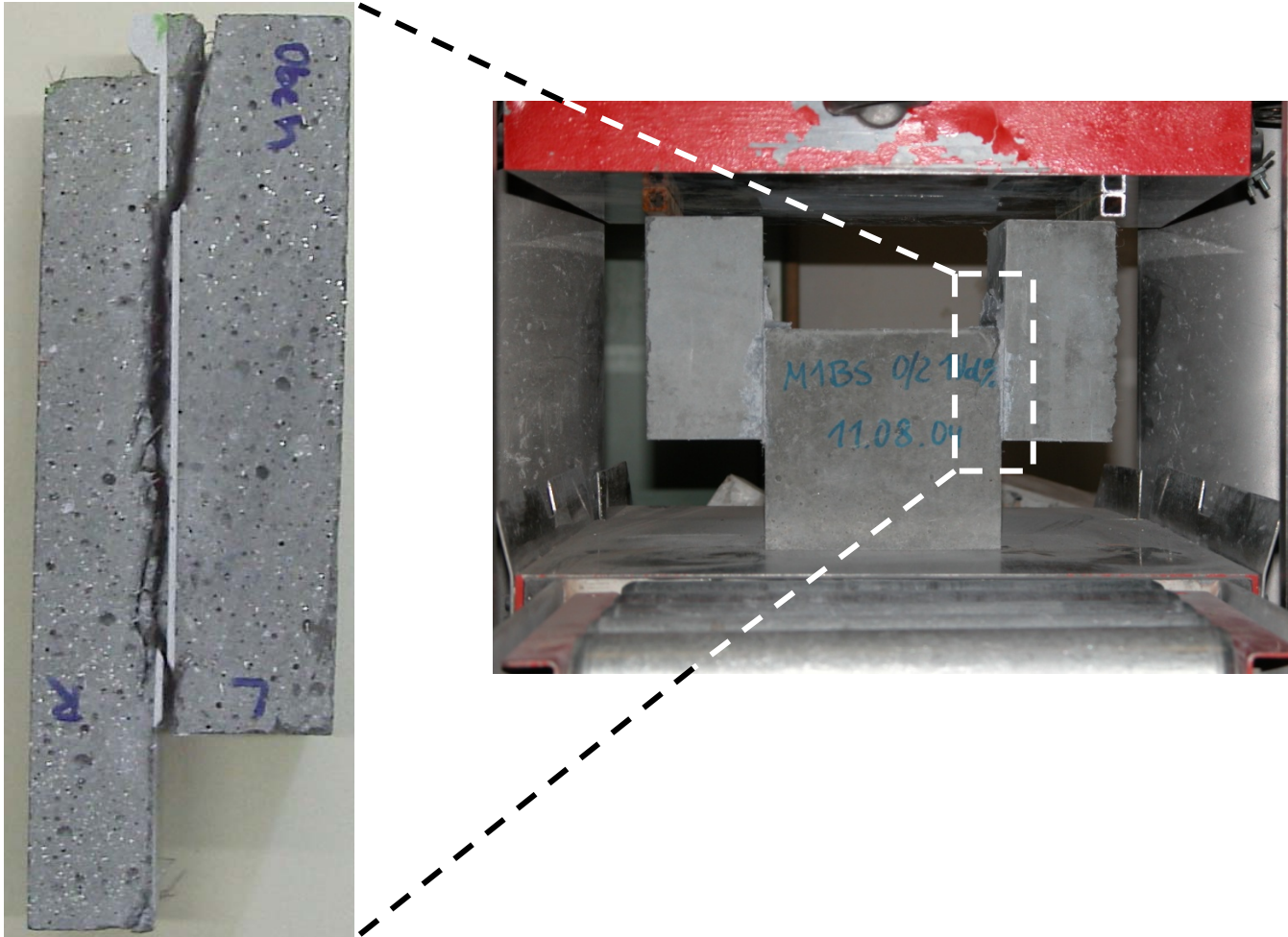
Crack spacing <<

Crack width <<



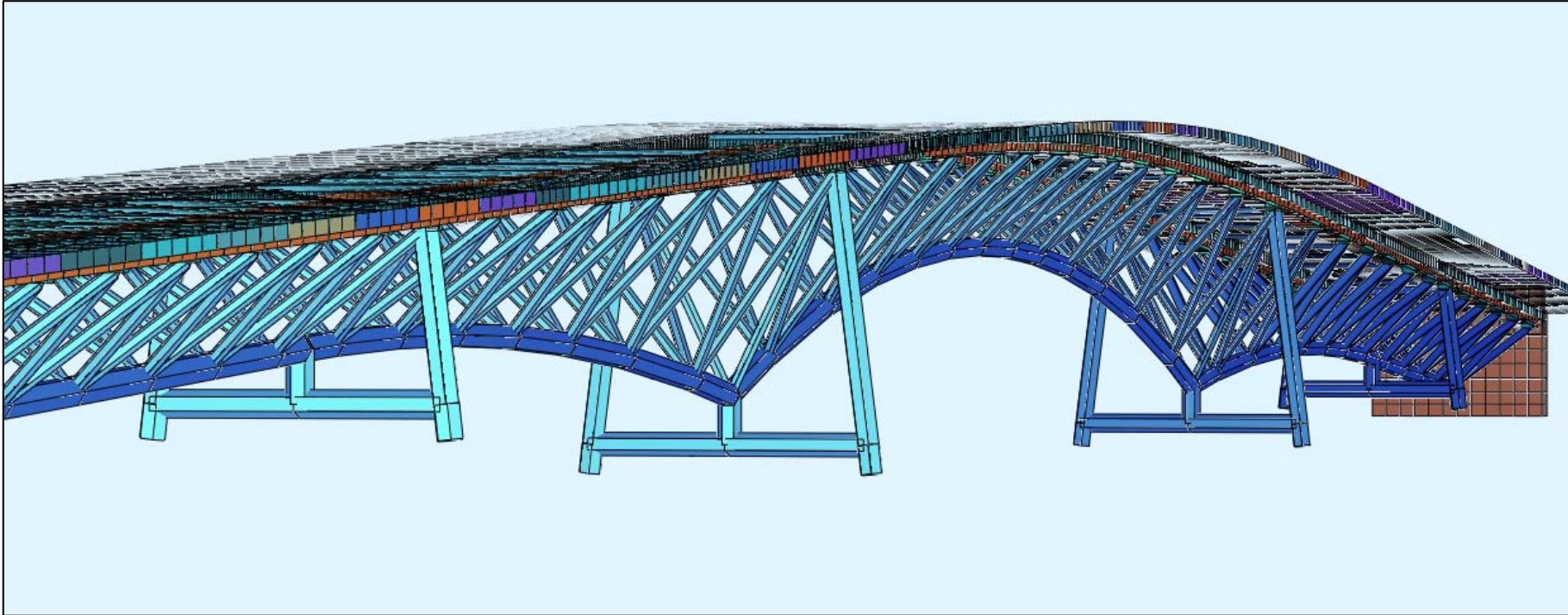
The Gärtnerplatz Bridge

Shear / Tension Test



The Gärtnerplatz Bridge

Structural Analysis



- SOFiSTiK software package
- Detailed considerations of construction sequence
- Effects of force redistributions due to creep (and shrinkage)

The Gärtnerplatz Bridge

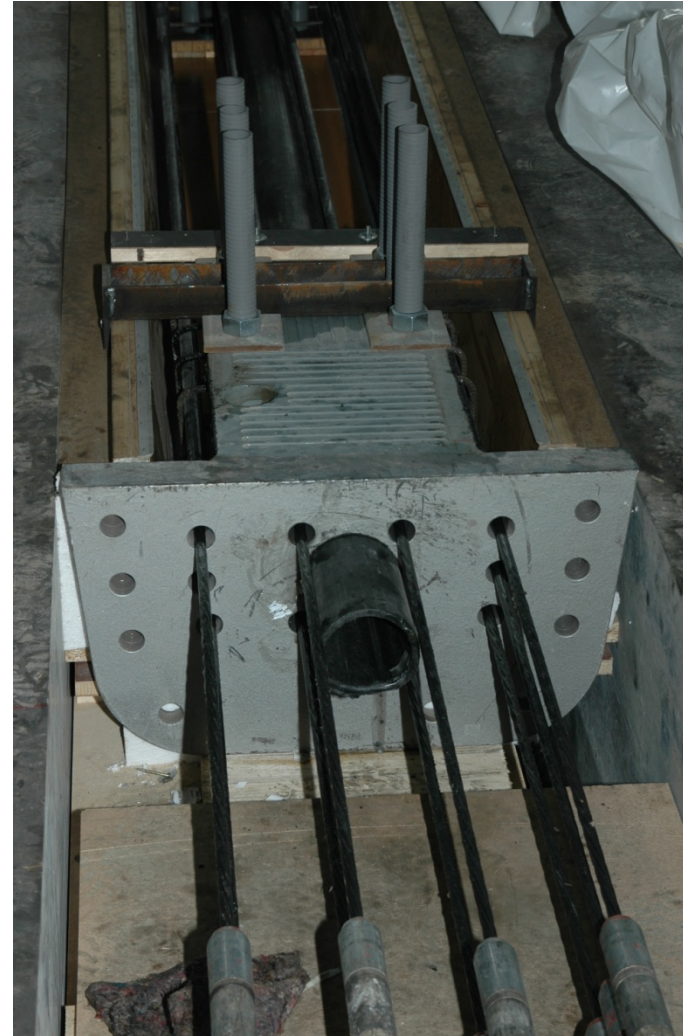
Precast UHPC elements for Bridge Decks



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The Gärtnerplatz Bridge

Precast UHPC Element for upper chords



The Gärtnerplatz Bridge

Connecting Steel Truss and upper chord



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The Gärtnerplatz Bridge

Transportation to site



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The Gärtnerplatz Bridge

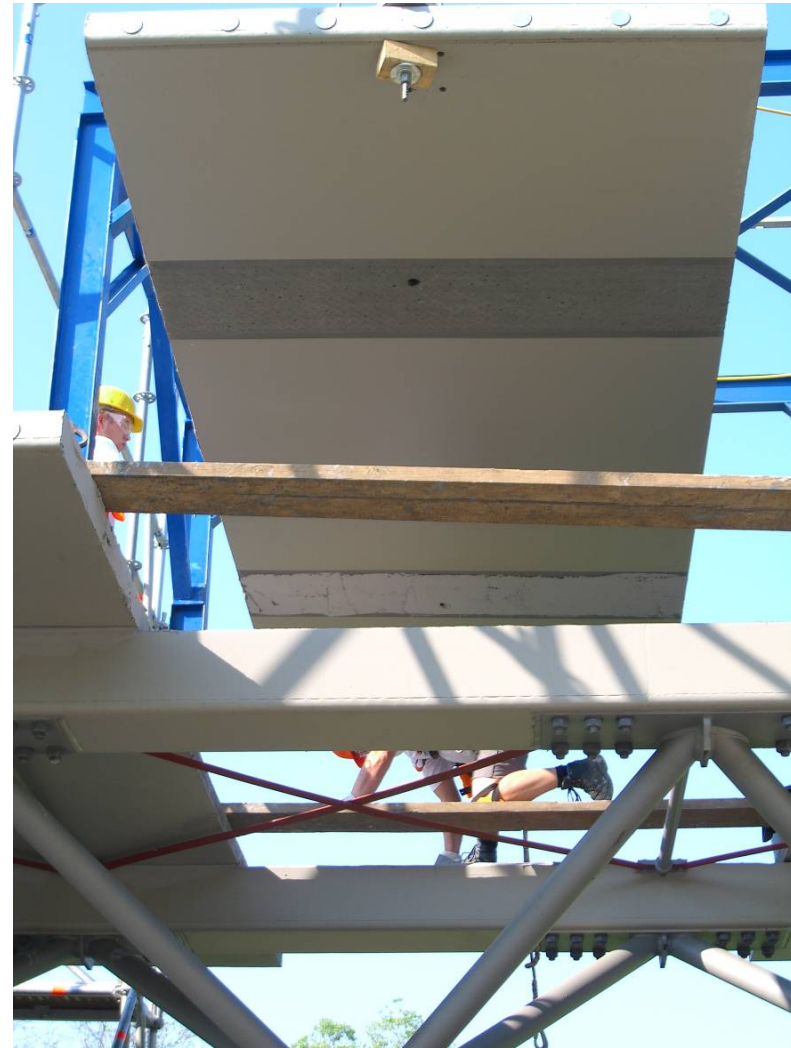
Connecting of superstructure to continuous beam



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The Gärtnerplatz Bridge

Assembling of bridge decks



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The Gärtnerplatz Bridge

Assembling of bridge decks



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The Gärtnerplatz Bridge

Assembling of handrails

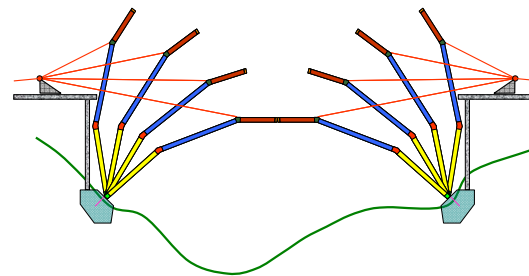
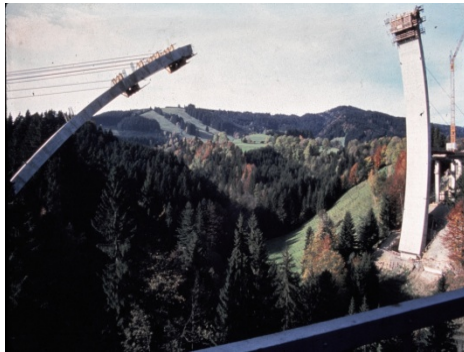


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The “WILD Bridge”, Austria

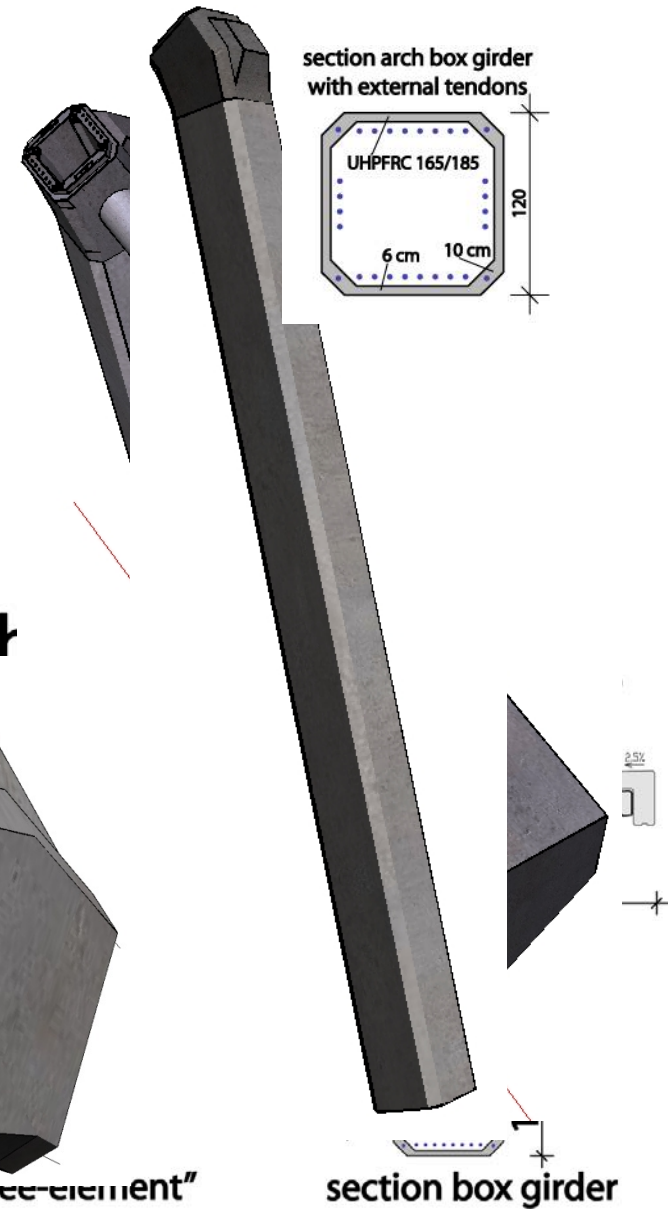
UHPC for Precast Segmental Arch Structure

Swivel with Method of Arch Erection



Erection and Elements

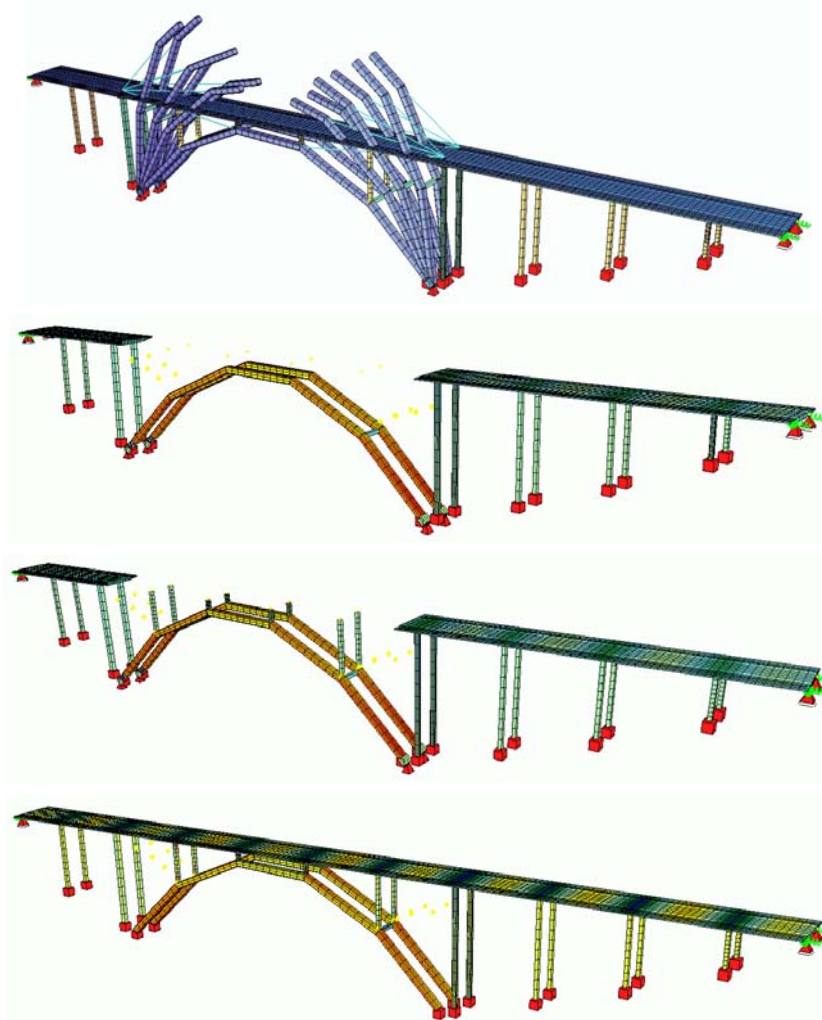
The “WILD Bridge”



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The “WILD Bridge”

Modelling, Analysis and Design



SLS/ULS Stress-strain relations for the design of cross-sections taken from German "Sachstandsbericht"

SLS:

- Decompression in the arch and the segmental joints under characteristic loads in longitudinal direction

adequate durability!!!!

- In transverse direction and in the „knee-elements“ the tension stresses should be lower than the matrix strength under the characteristic loads

ULS:

- Segments; stress-strain relation for tension and compression, segmental joints; only compression

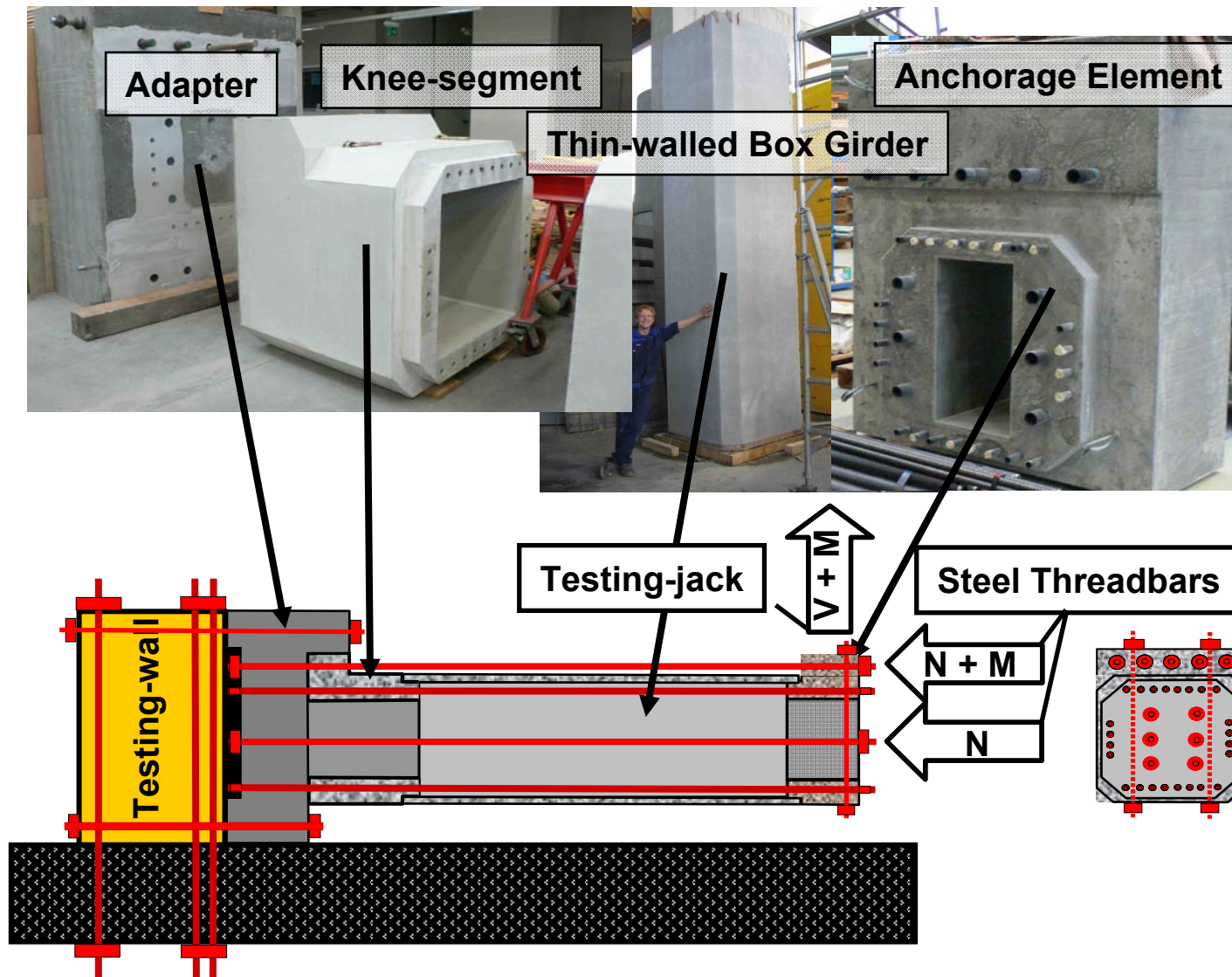
- Gaping of the segmental joints is limited by one third of the height of the section

- Low shear force easily transferred in the remaining compression zone through friction

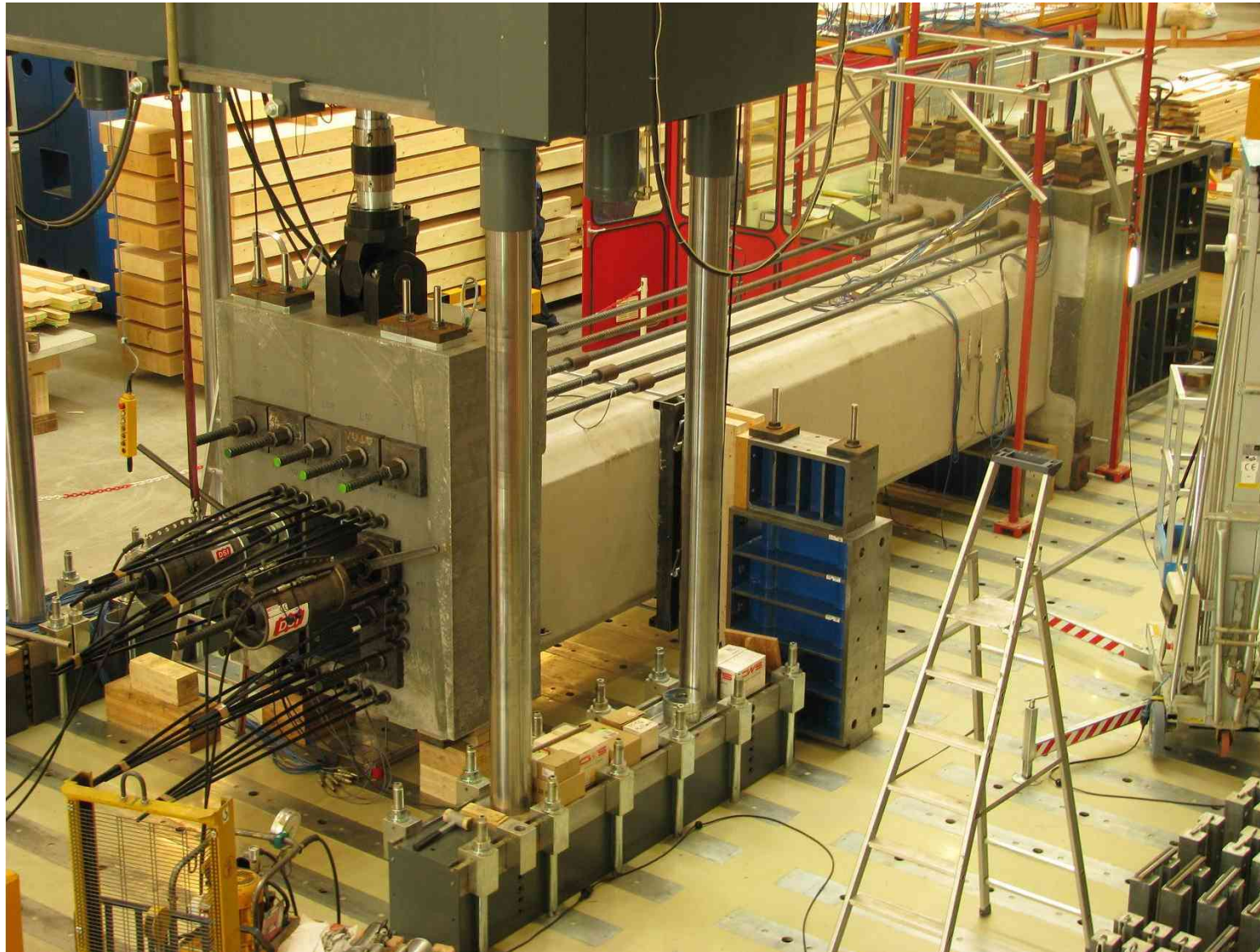
- Shear carrying capacity calculated according to German guideline „Stahlfaserbeton“

The “WILD Bridge”

1 : 1 Full Scale Test

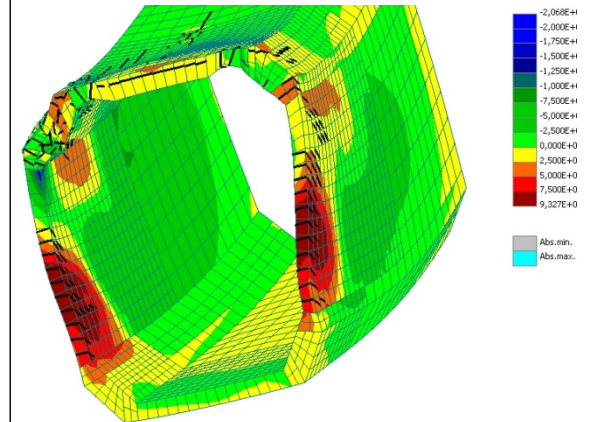
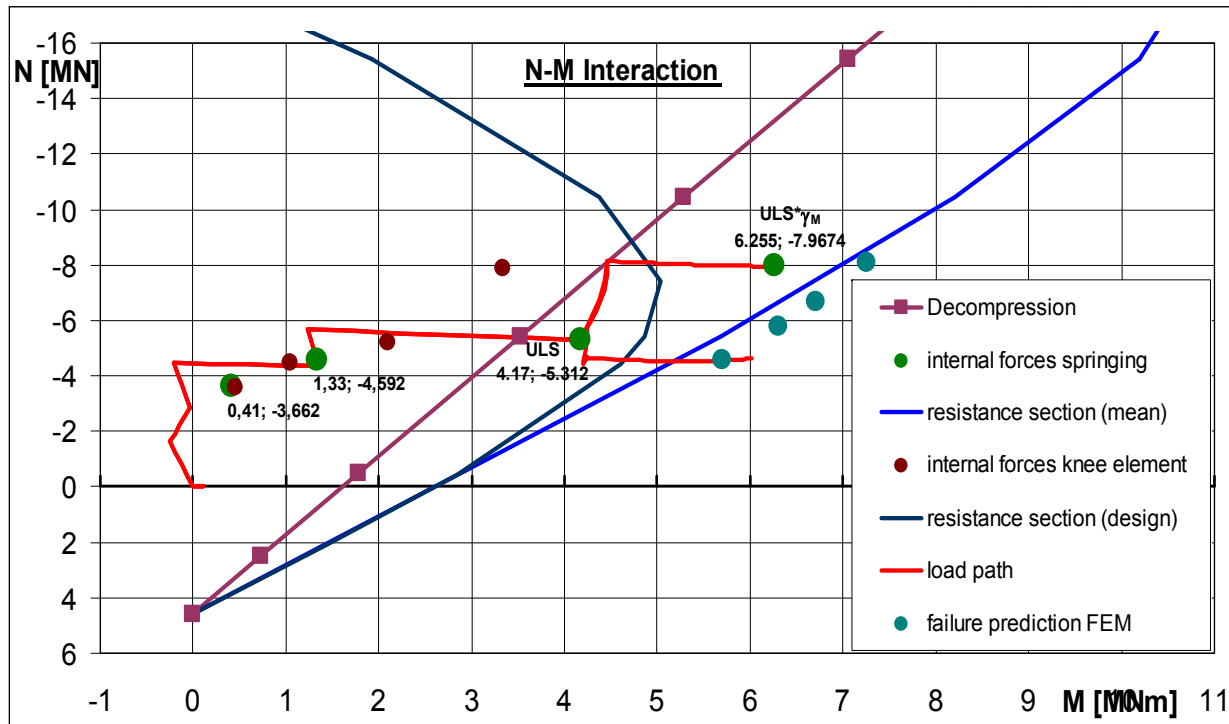
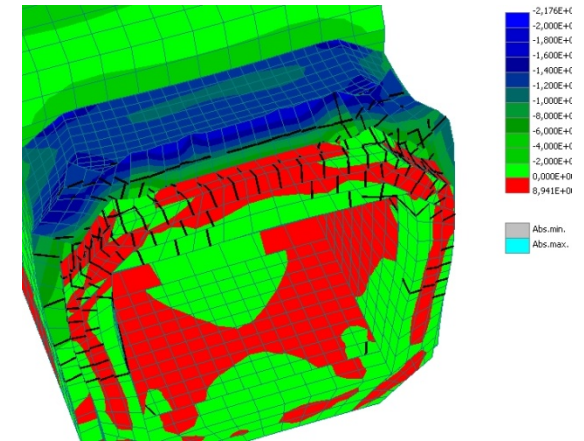
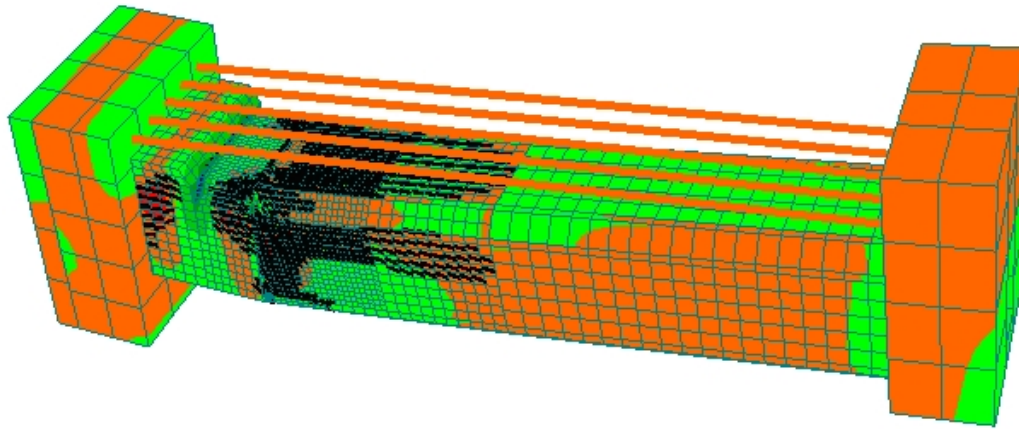


The “WILD Bridge”



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The “WILD Bridge”



The “WILD Bridge”



The “WILD Bridge”

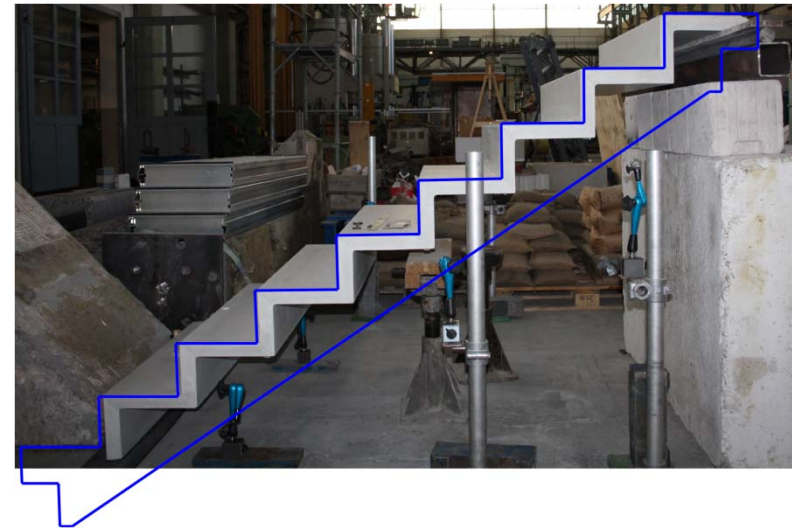


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UHPC – Stair without Conventional Reinforcement



Comparison



	Conventional stair	UHPC- stair
Concrete volume	0,850 m ³	0,205 m ³
Mass	2.125 kg	515 kg

**Thanks for your
kind attention!**

