

CIVIL 449: Nonlinear Analysis of Structures

School of Architecture, Civil & Environmental Engineering
Civil Engineering Institute

Case Studies on the Use of Nonlinear Analysis

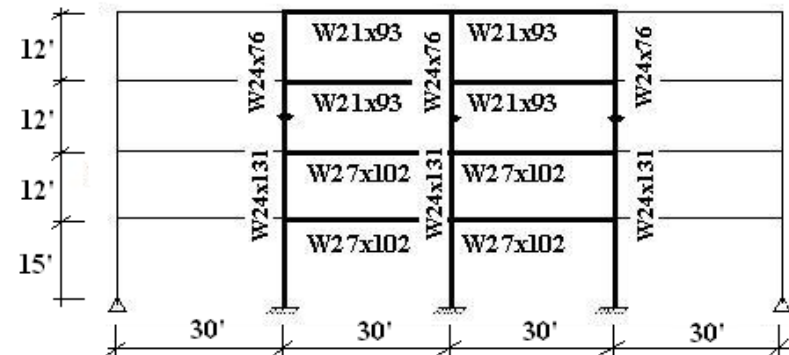
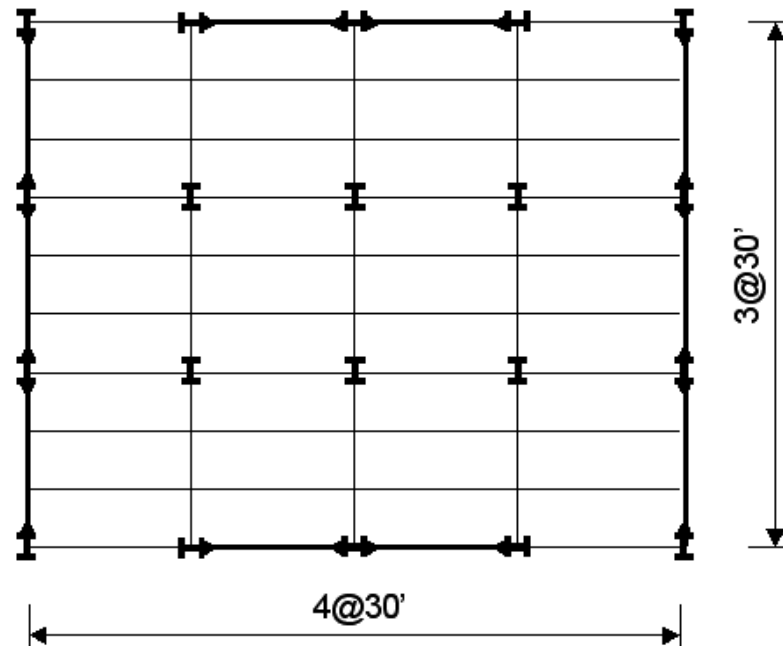
Prof. Dr. Dimitrios Lignos
EPFL, ENAC, IIC, RESSLab

EPFL Objectives of today's lecture

- “Blind” nonlinear analysis of a 4-story steel building
 - archives from a previous nonlinear analysis course
- Case study #1: Prediction of nonlinear response of a 4-story steel frame building tested at full-scale through collapse
- Case study #2: Seismic retrofit of existing structures
 - Application to an existing hospital building
- Concluding remarks

EPFL 4-story steel building with moment-resisting frames

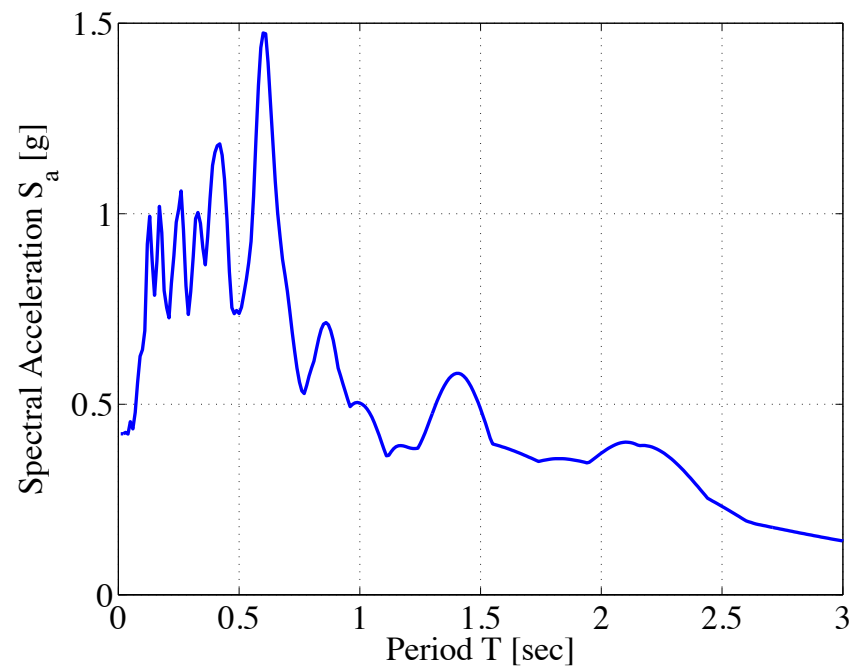
- Archives: Nonlinear Analysis for Buildings
- McGill University, Quebec, Canada / Instructor: D.Lignos (2014)



Source: Lignos et al. (2011)

EPFL 4-story steel building with moment-resisting frames

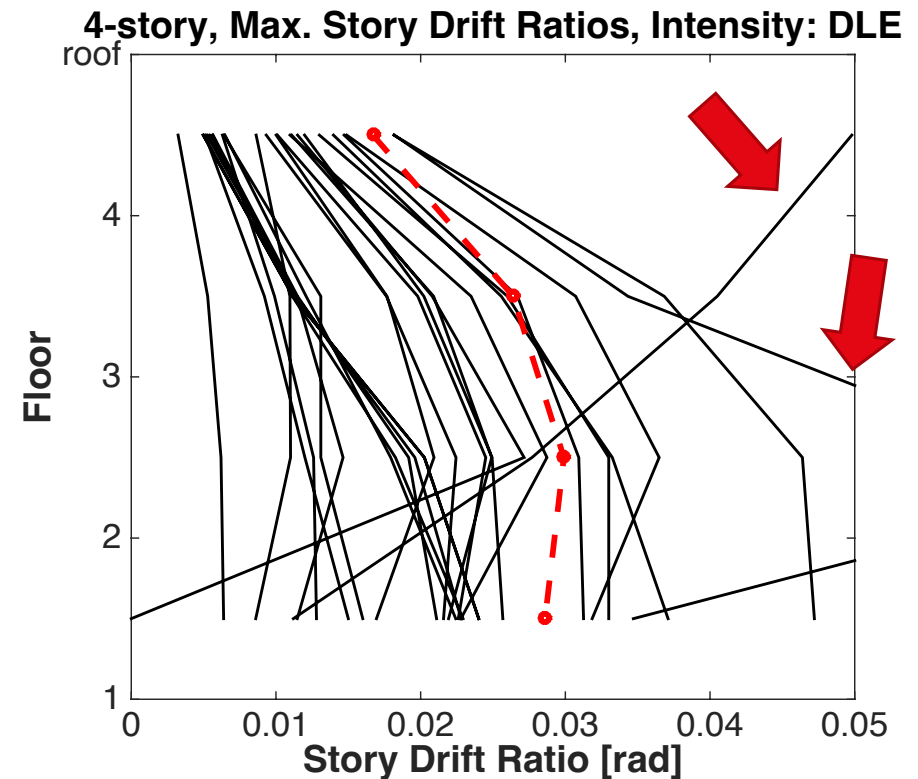
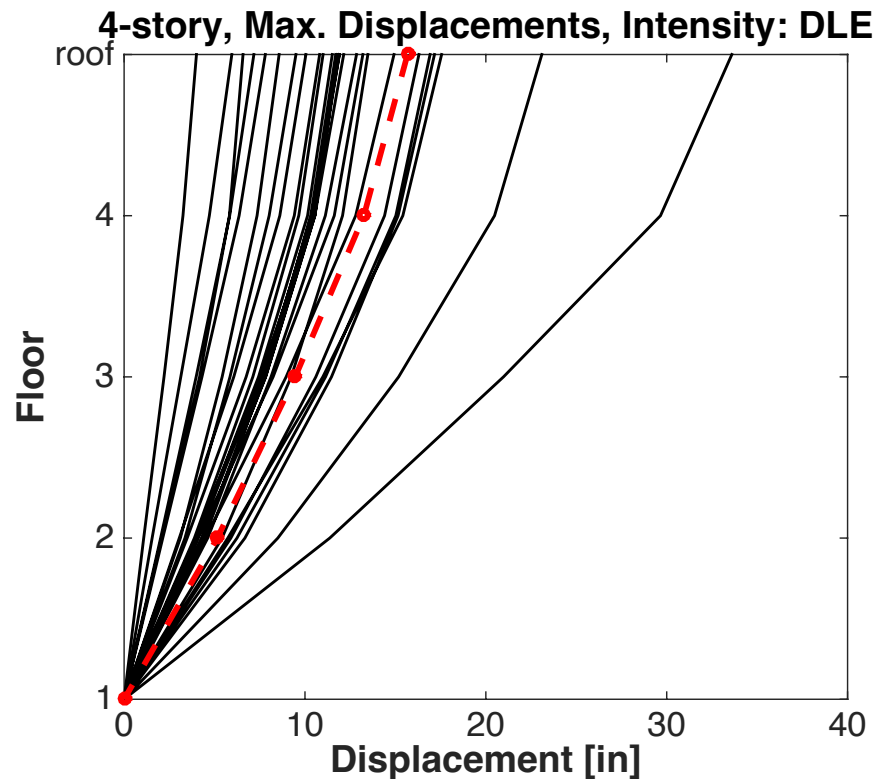
- Canoga Park record from 1994 Northridge earthquake
 - Los Angeles, California, USA



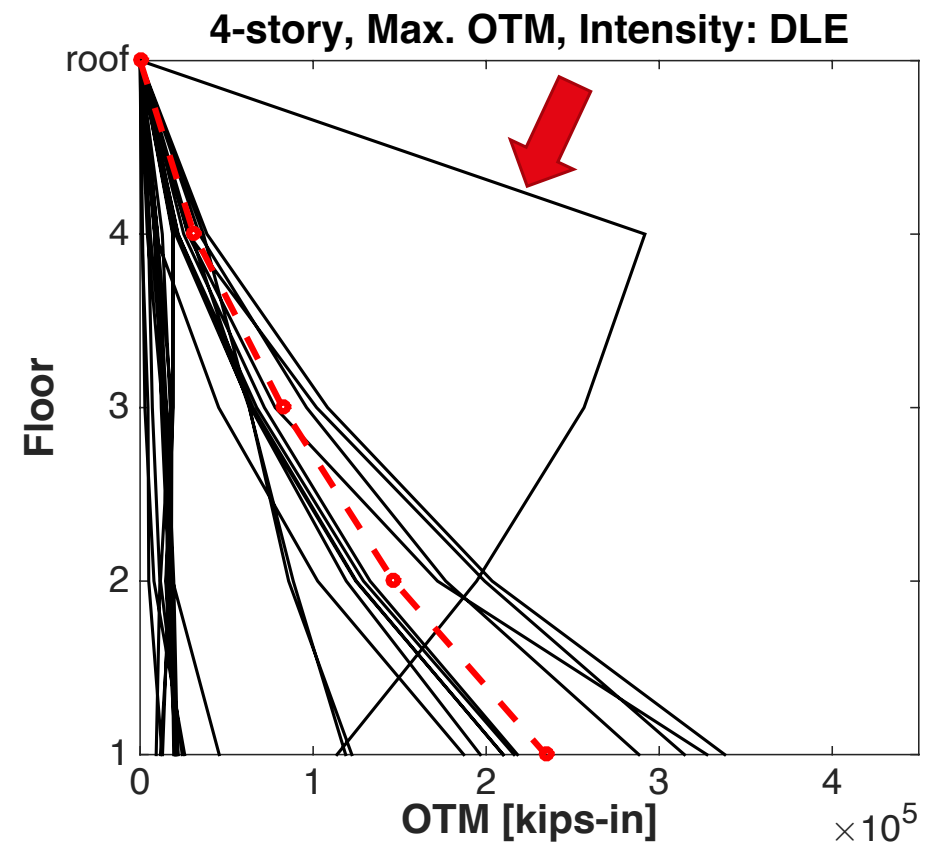
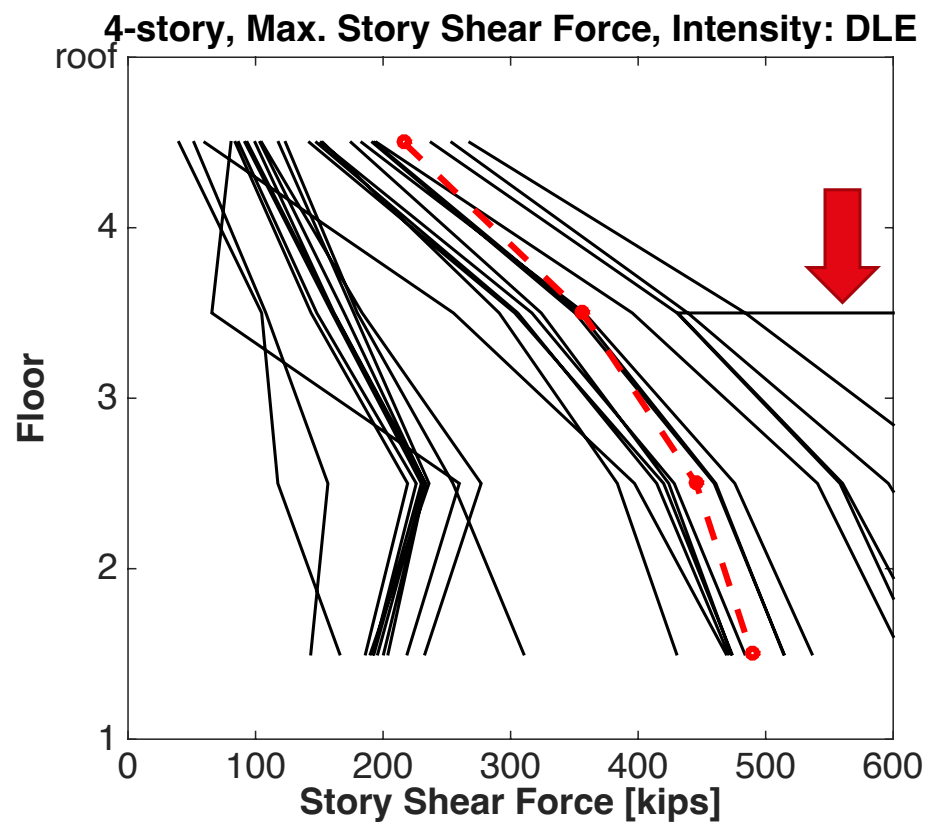
EPFL 4-story steel building with moment-resisting frames

- 30 registered students
- Taught by the same instructor (Me 😊)
- Free to use any nonlinear analysis software and type of model
- Nonlinear analysis at 3 different target displacements
- Were given a spreadsheet to fill with their responses
- Questions: peak displacements, peak story drift angle, peak story shear forces and peak overturning moments (OTM) at each floor of the frame

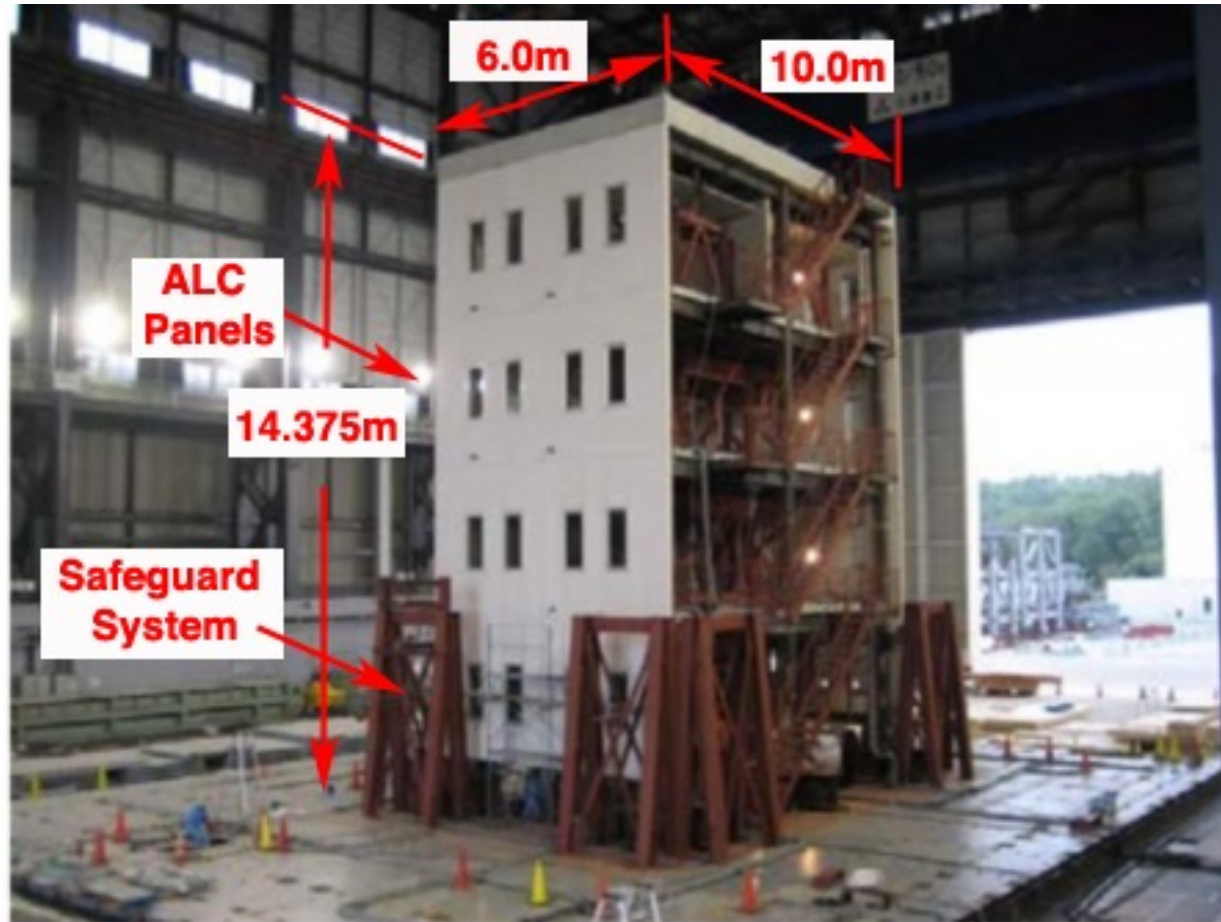
4-story steel building with moment-resisting frames



4-story steel building with moment-resisting frames

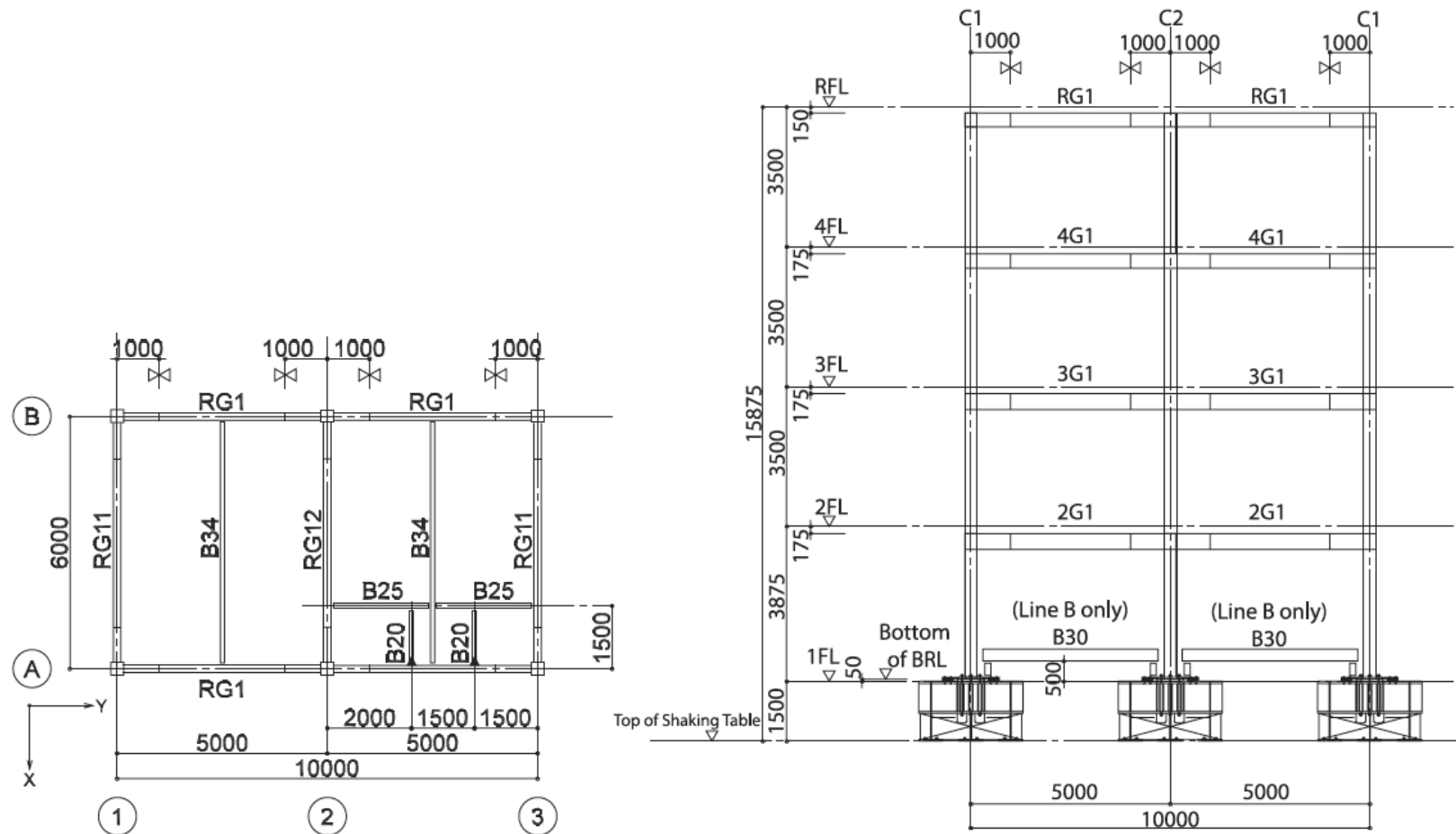


4-Story steel building tested at E-Defense



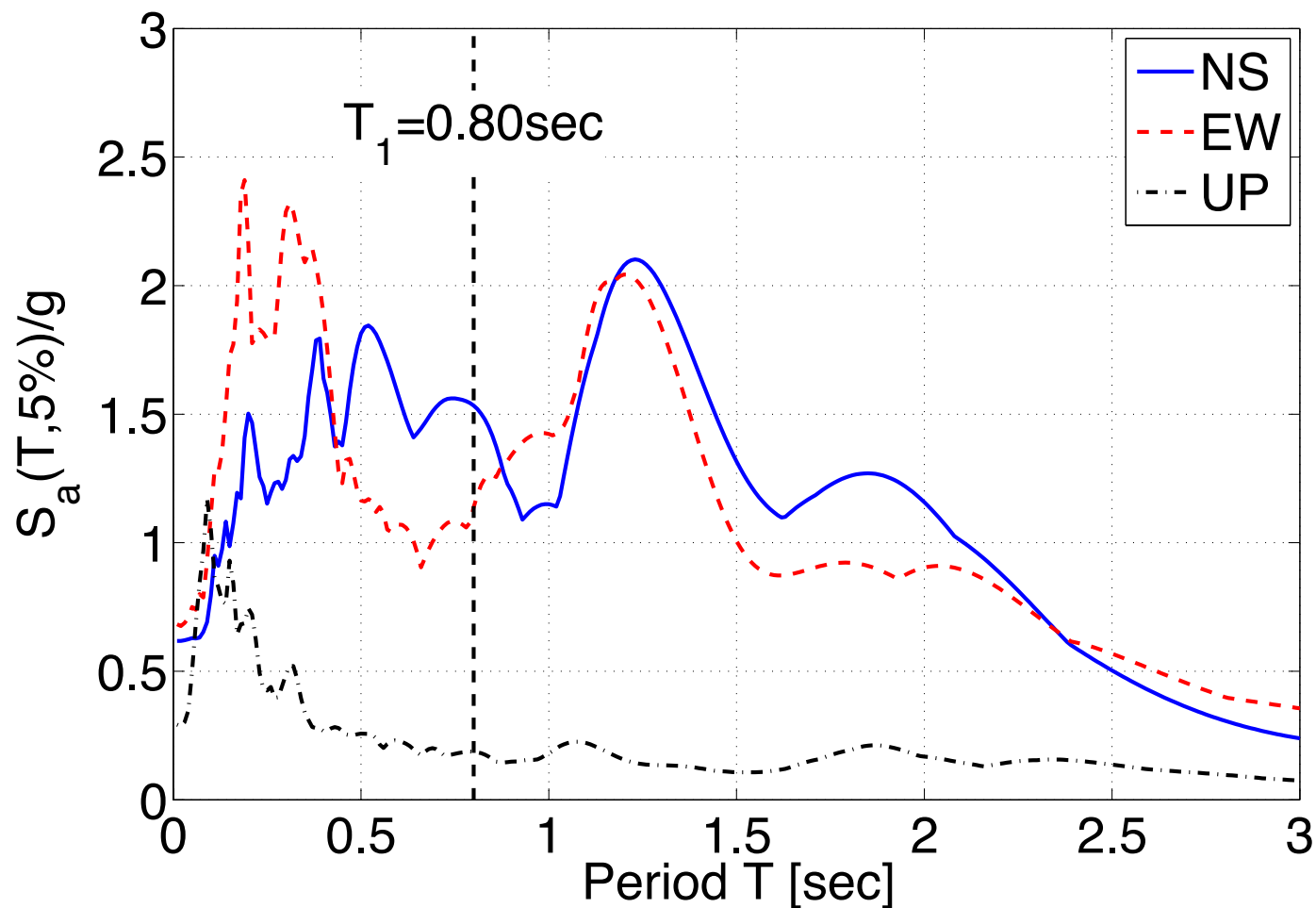
E-Defense, Japan, 2007
Image from Lignos et al. (2013)

4-Story steel building tested at E-Defense



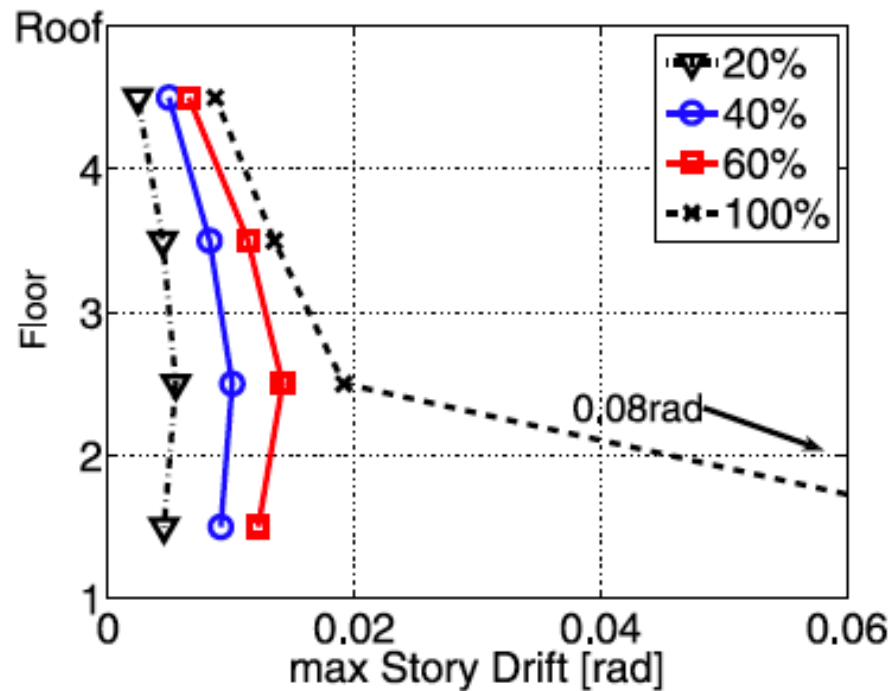
Source: Lignos et al. (2013)

4-Story steel building tested at E-Defense

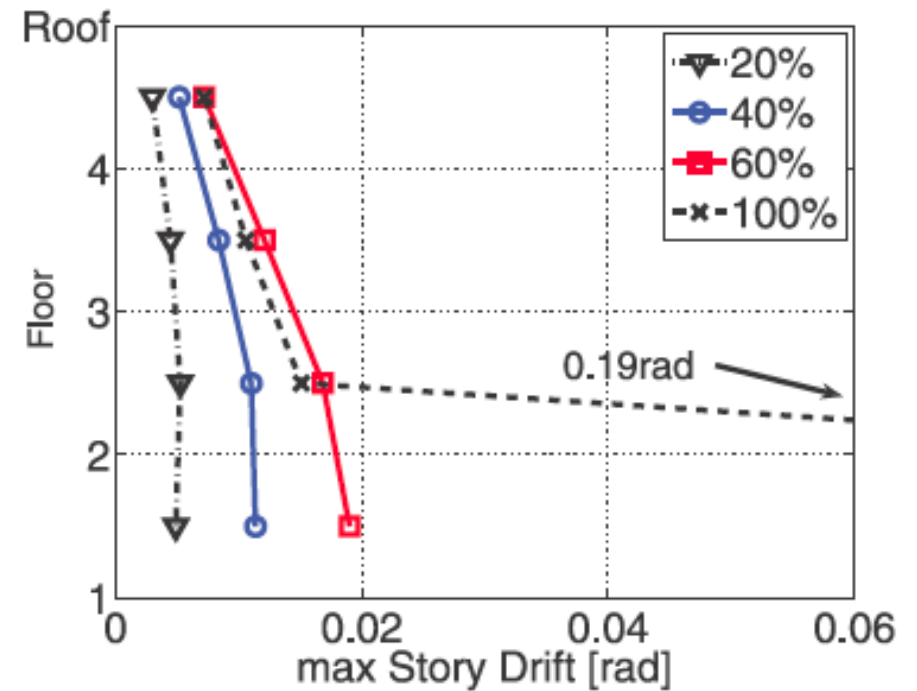


Source: Lignos et al. (2013)

4-Story steel building tested at E-Defense



(a) X-loading direction



(b) Y-loading direction

4-Story steel building tested at E-Defense



4-Story steel building tested at E-Defense



- Collapse was attributed to:
 - Composite effects (increase the beam flexural strength)
 - Flexibility of column bases
 - Steel material variability (columns versus beams)
 - 3D effects (N-Mx-My interaction)
 - Column plastic hinging at the base causes strength deterioration and inflection point shifting

Source: Lignos et al. (2013)

EPFL International blind analysis contest

- Winner is selected for each of the four categories:
 - Category 1: 3D Analysis (Researcher)
 - Category 2: 3D Analysis (Practicing Engineer)
 - Category 3: 2D Analysis (Researcher)
 - Category 4: 2D Analysis (Practicing Engineer)

EPFL International blind analysis contest

- 47 teams from 7 countries in total
- 7 teams participated in both 3D and 2D analysis

Country \ Category	3D-R	3D-P	2D-R	2D-P	TOTAL
Japan	6	5	4	2	17
U.S.	6	5	2	2	15
Taiwan	4	0	4	0	8
China	1	1	2	0	4
N.Z.	0	1	0	0	1
Italy	1	0	0	0	1
U.K.	0	0	0	1	1
TOTAL	18	12	12	5	47

3D : Three-dimensional Frame Analysis 2D : Plane Frame Analysis

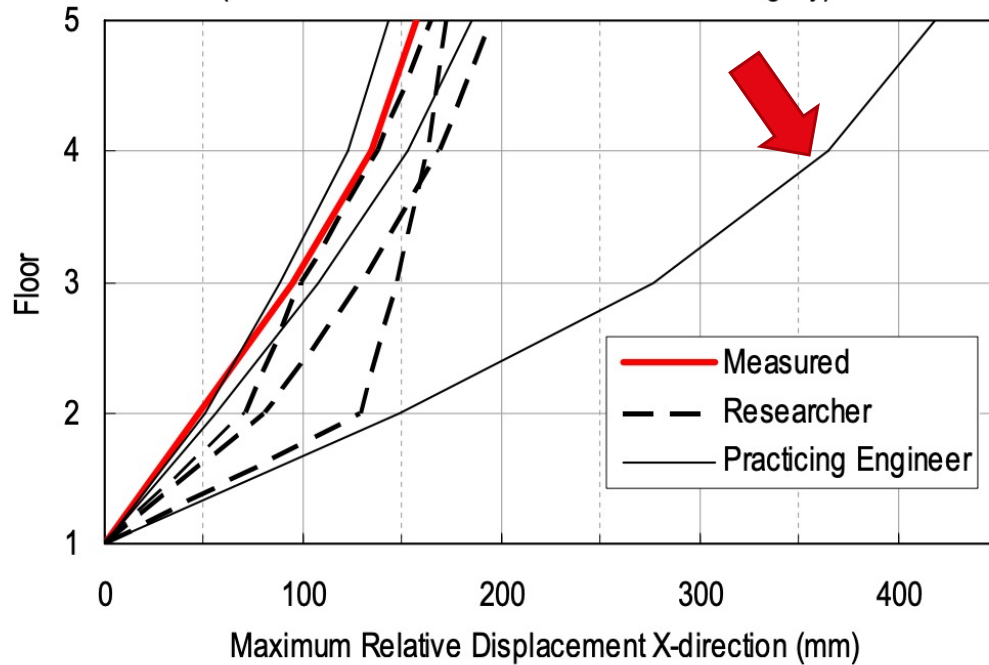
R : Researcher

P : Practicing Engineer

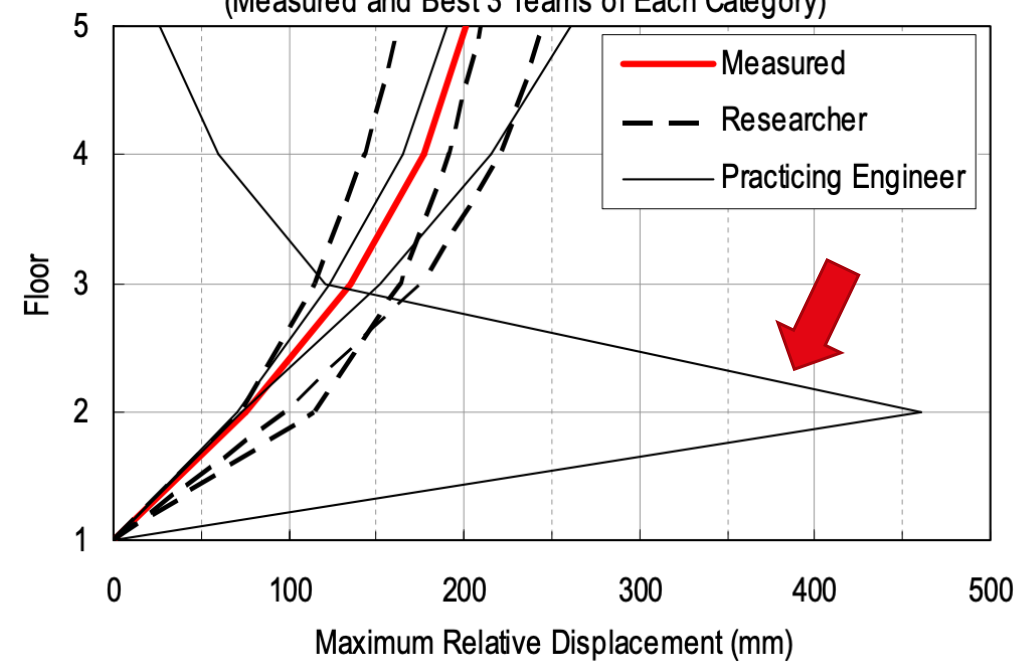
Table Source: E-Defense 2007

International blind analysis contest

3D Analysis Blind Prediction Results
Maximum Relative Displacement X-direction
(Measured and Best 3 Teams of Each Category)

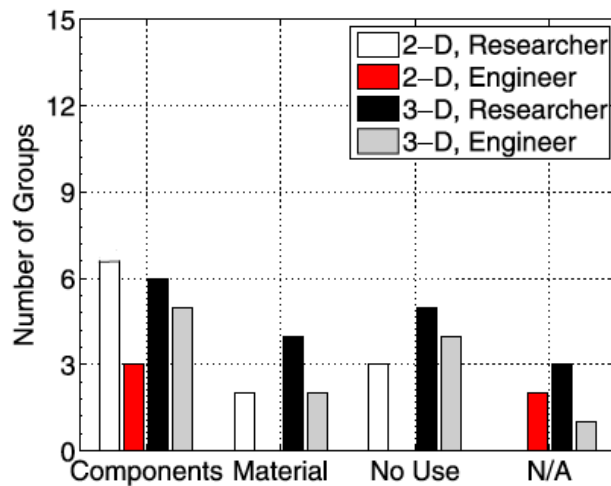


2D Analysis Blind Prediction Results
Maximum Relative Displacement
(Measured and Best 3 Teams of Each Category)

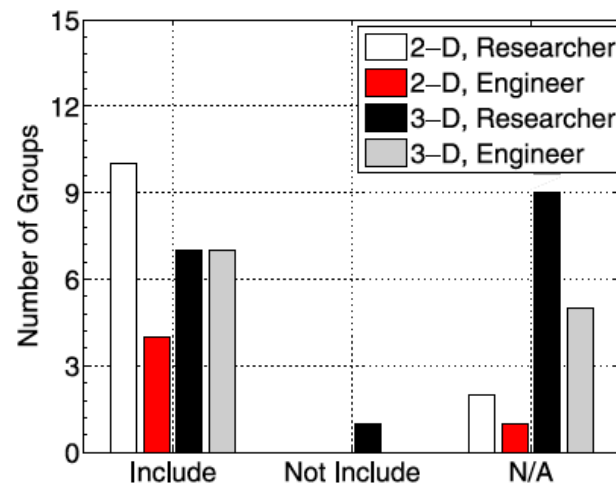


EPFL International blind analysis contest

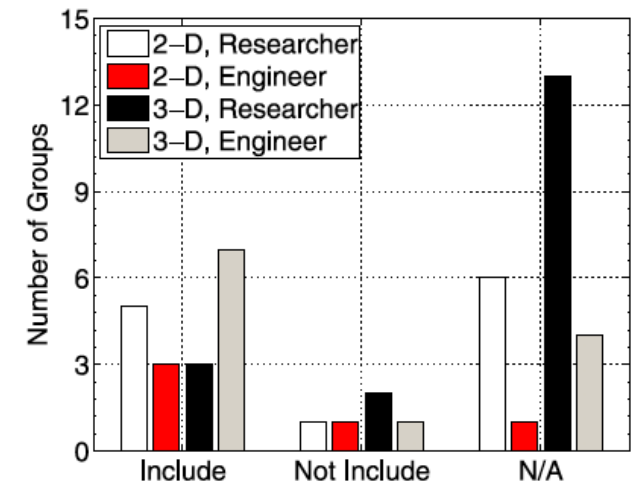
- Some key findings regarding the modelling assumptions of the participants



(a) Material & component tests



(d) Composite slab effects



(e) P-Delta effects

EPFL Modeling strategies

- Zero length spring elements
- Distributed plasticity

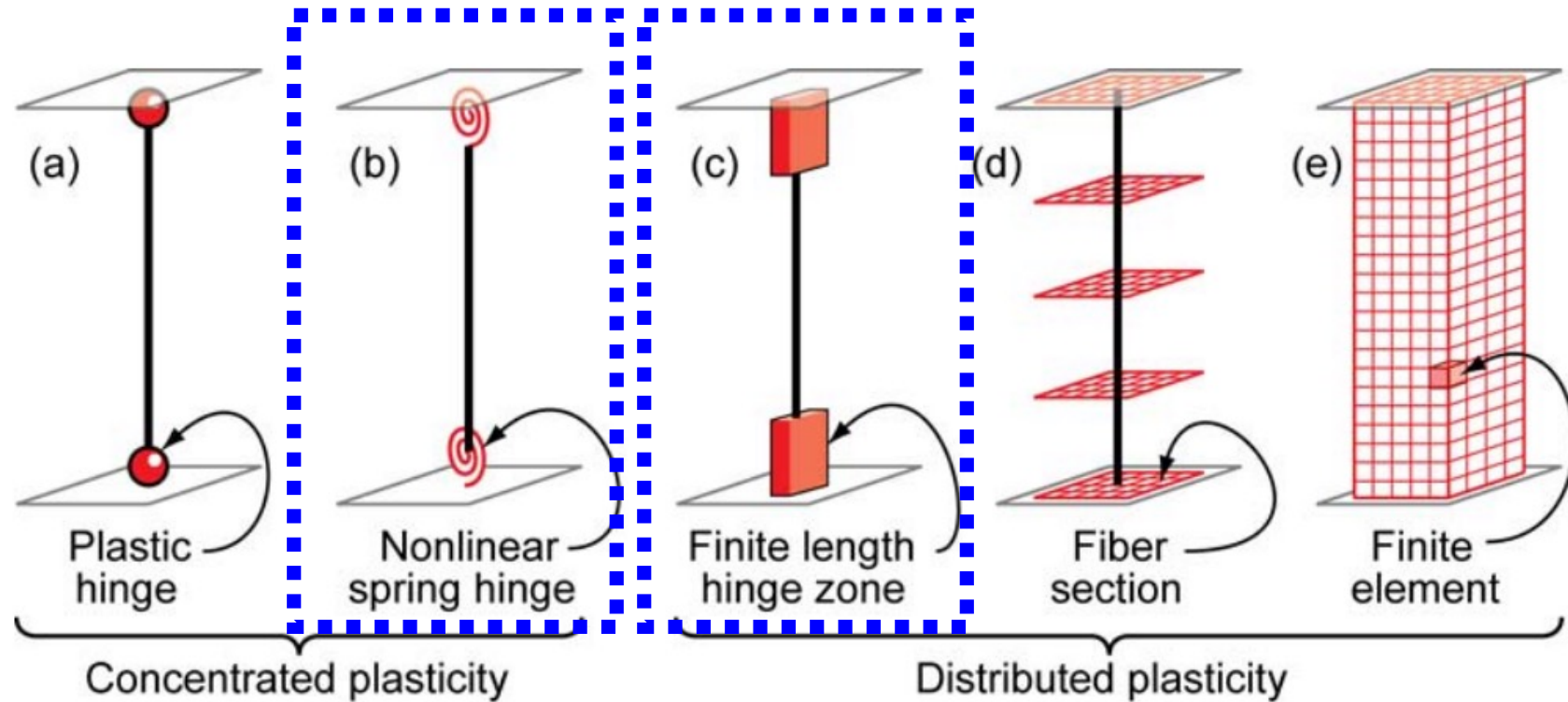
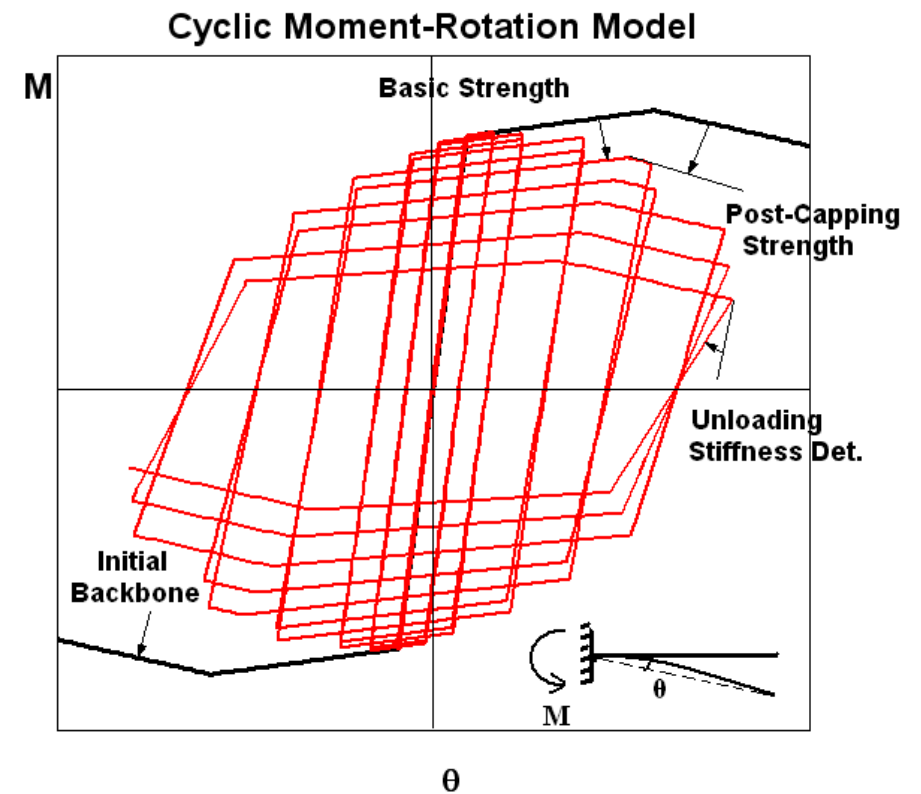
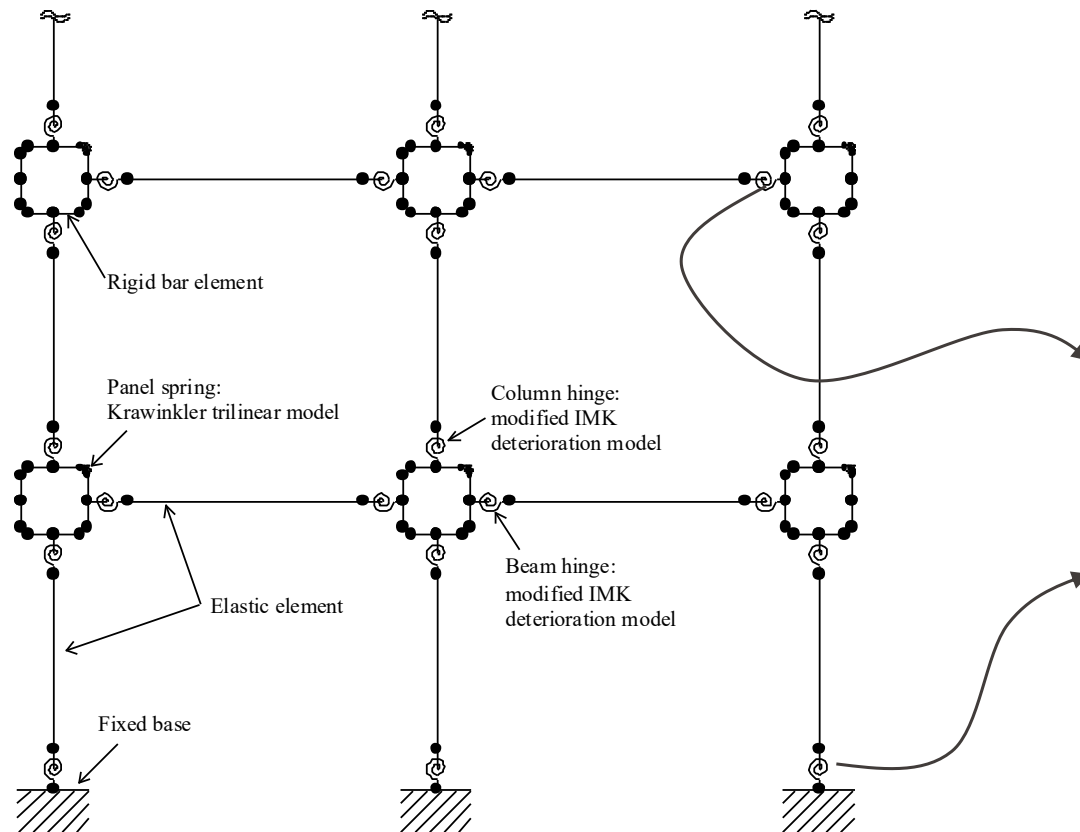


Image Source: NIST GSR 10-917-5

EPFL Modeling strategies

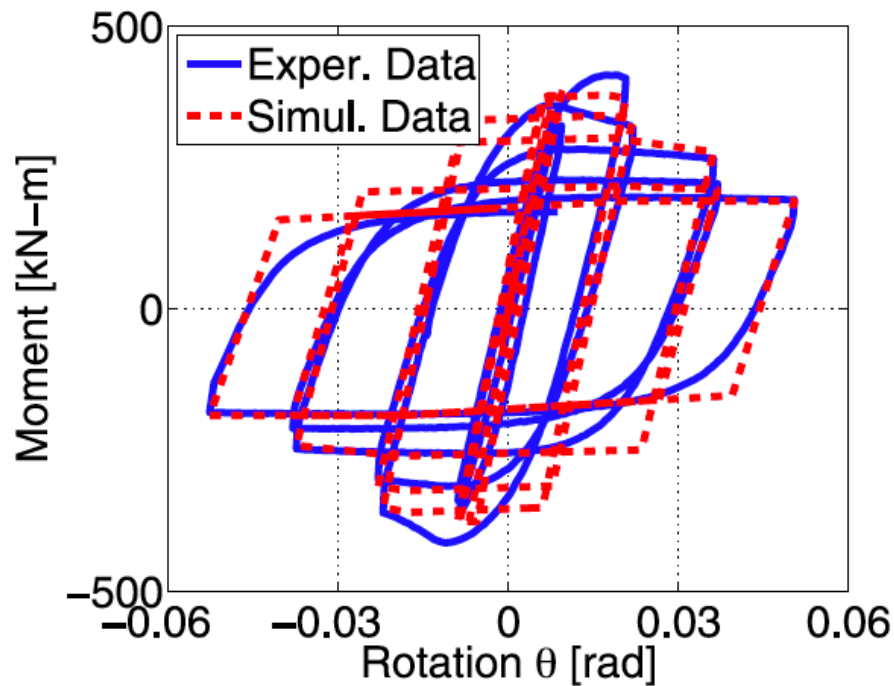
■ Zero length spring elements



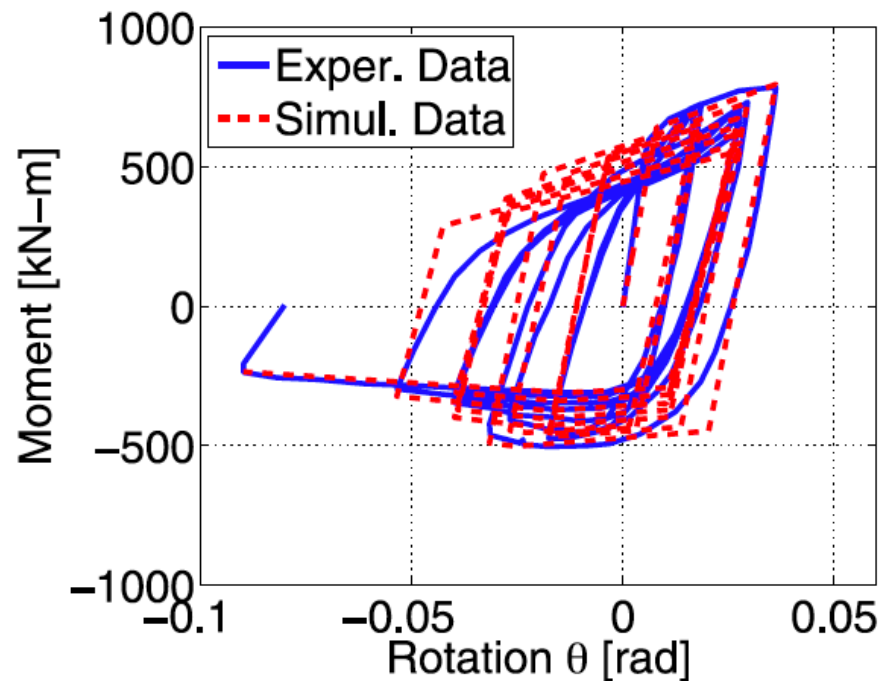
(Source: Lignos and Krawinkler 2011)

EPFL Modeling strategies

- Zero length spring elements



(a) HSS300x9 column tested in 45°

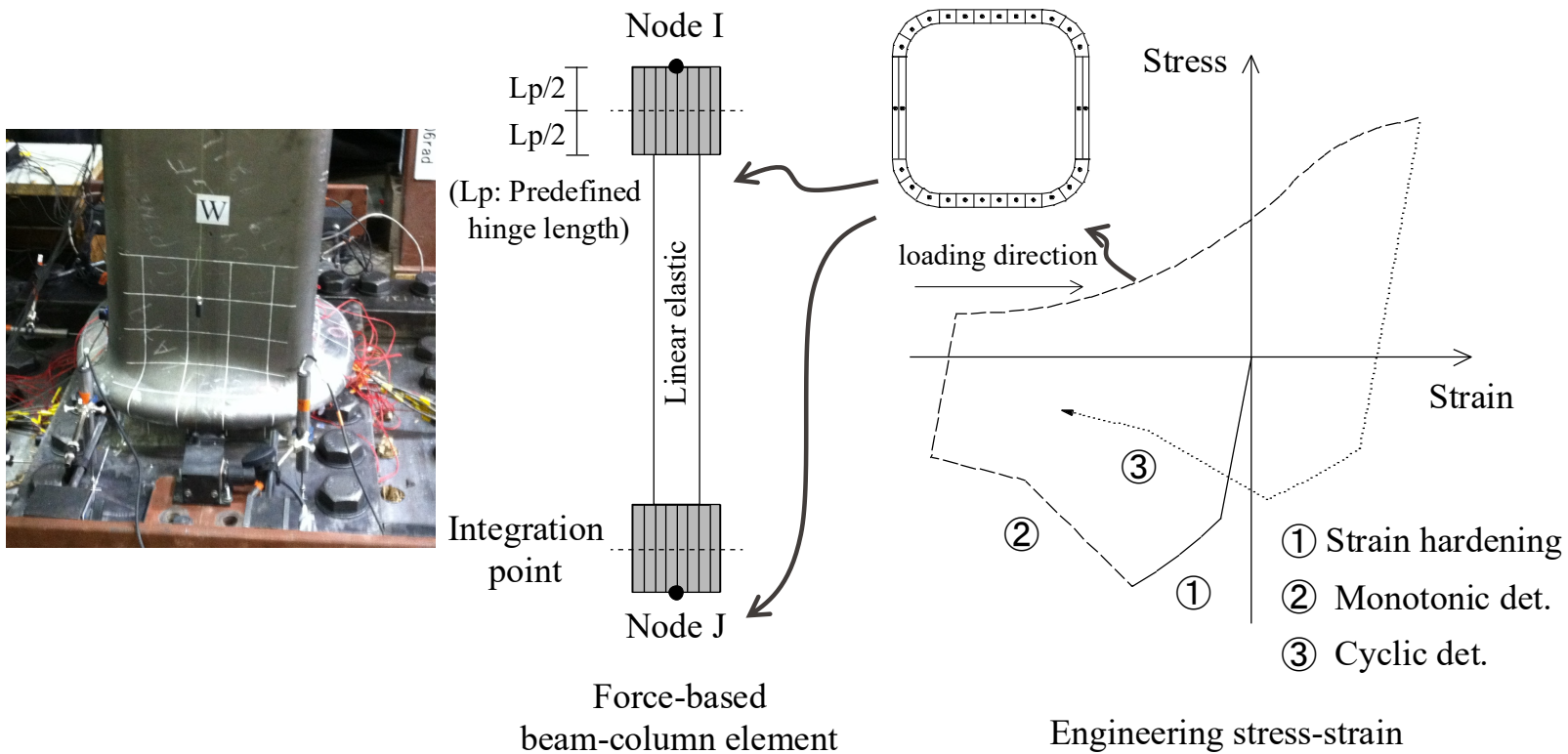


(b) H-400x200x8x13 composite beam

Source: Lignos et al. (2013)

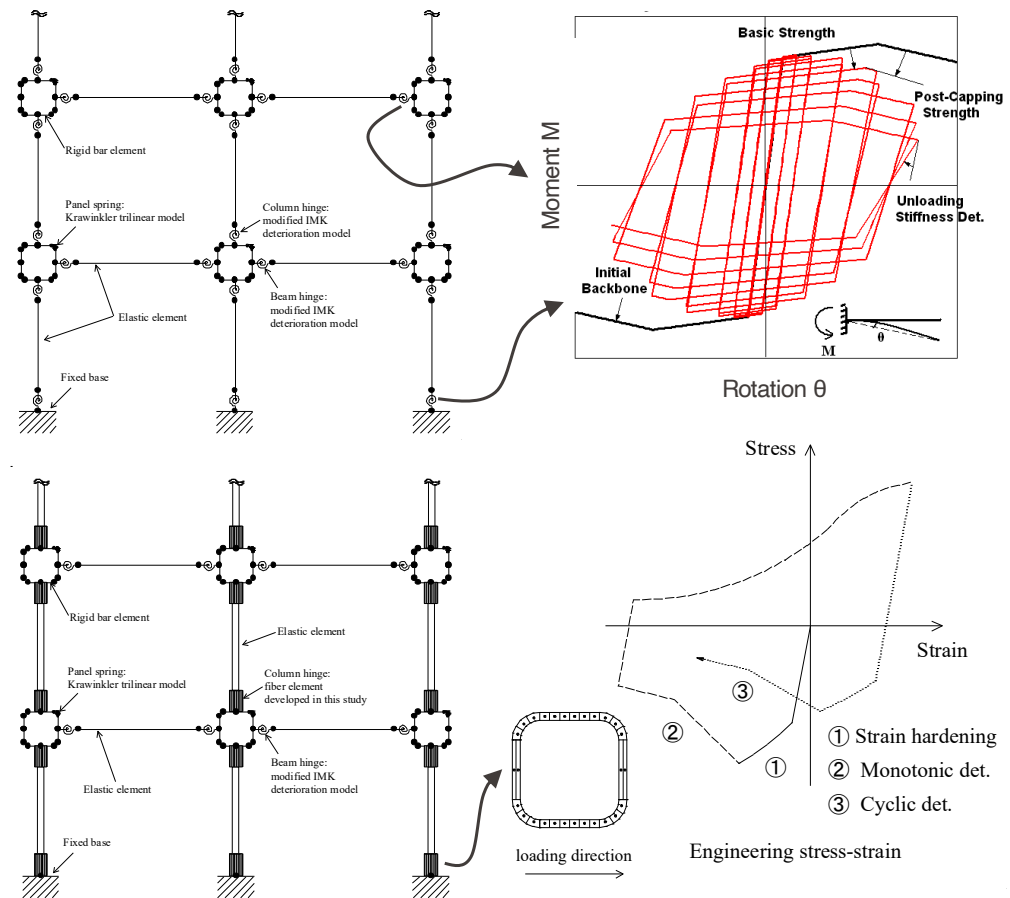
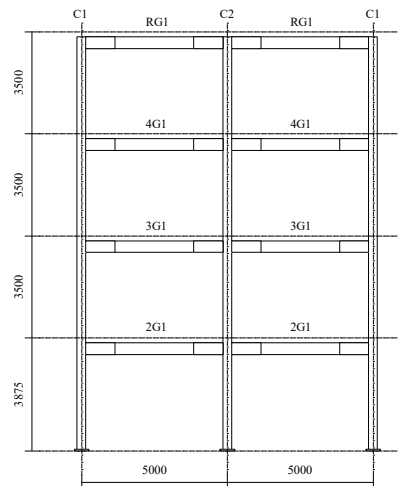
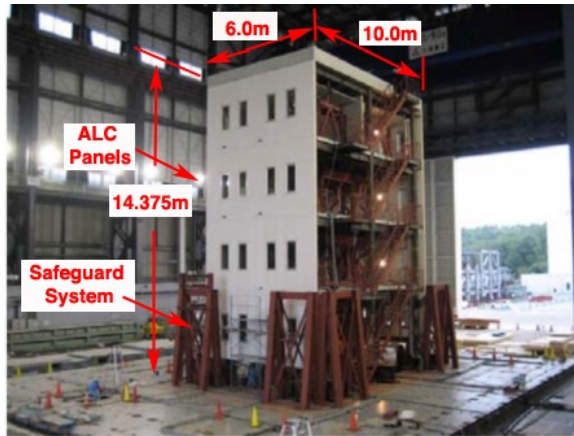
EPFL Modeling strategies

■ Fiber—based force-based element



Source: Suzuki and Lignos (2021)*

Modeling strategies

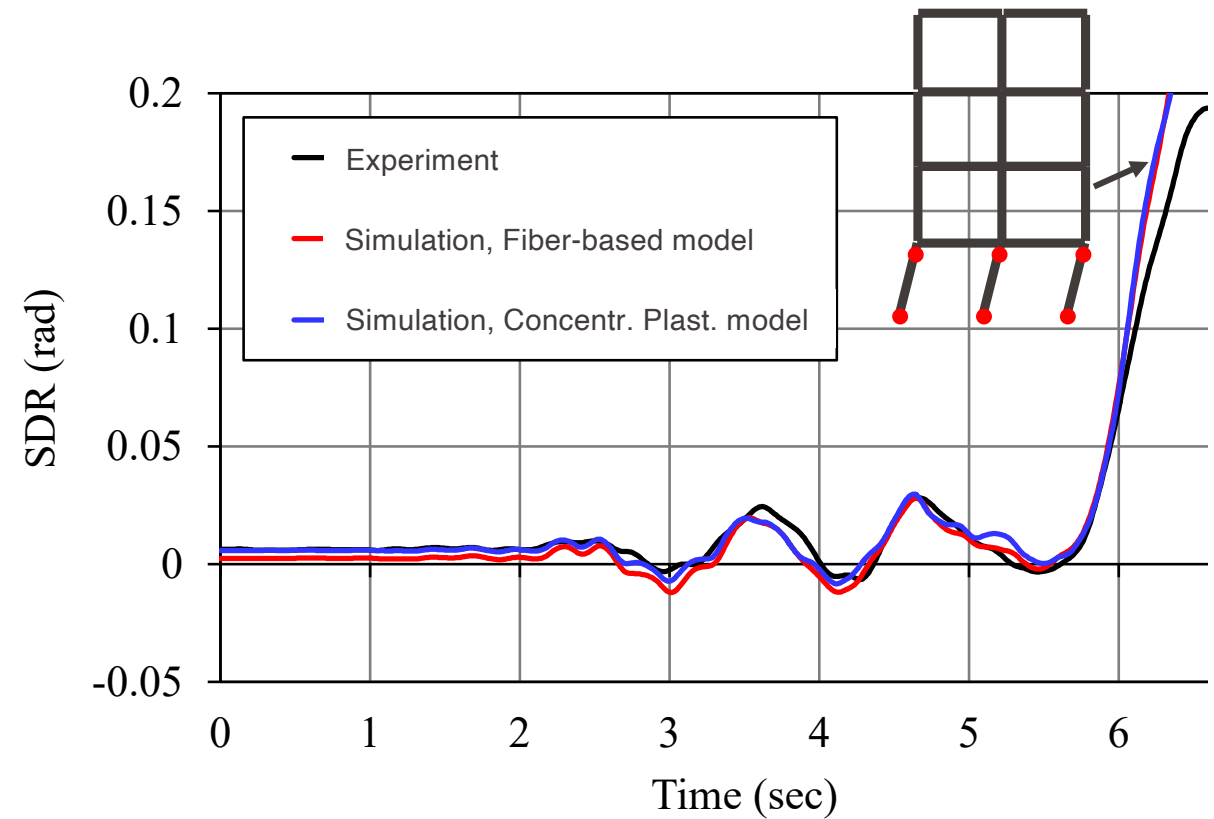


Source: Suzuki and Lignos (2018)*

EPFL Predicted response



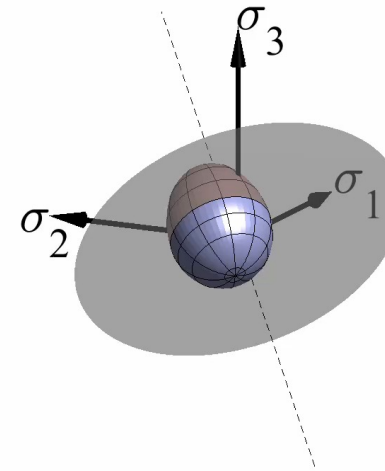
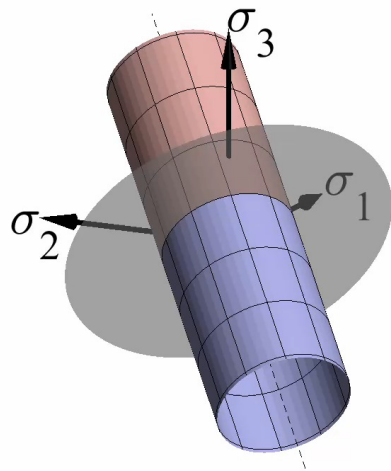
Prediction of 1st story collapse mechanism



Source: Suzuki and Lignos (2021)*

3-dimensional beam-column elements

-Multiaxial plasticity formulation

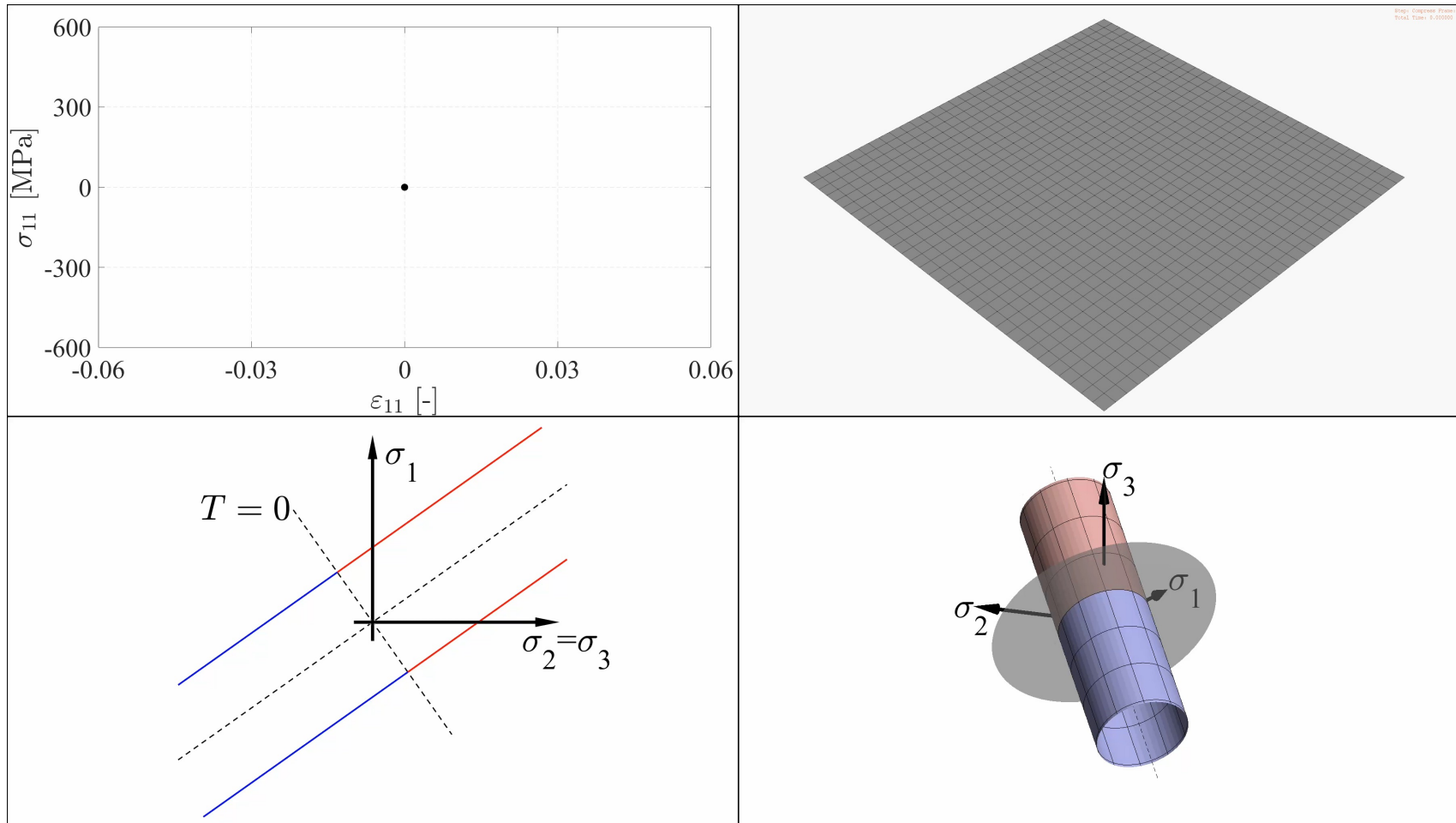


Yield surface

$$\phi = \begin{cases} \phi^{Tens}: \frac{3}{2} \xi^T \mathbf{P} \xi + \chi_{1t} (\mathbf{p}^T \boldsymbol{\sigma})^2 - \sigma_y^2 \leq 0, & T \geq 0 \\ \phi^{Comp}: \frac{3}{2} \xi^T \mathbf{P} \xi + \chi_{1c} (\mathbf{p}^T \boldsymbol{\sigma})^2 - \sigma_y^2 \leq 0, & T \leq 0 \end{cases}$$

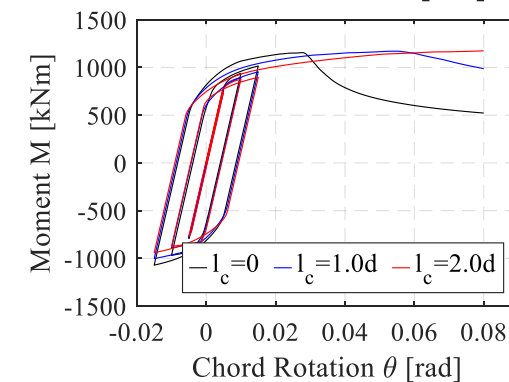
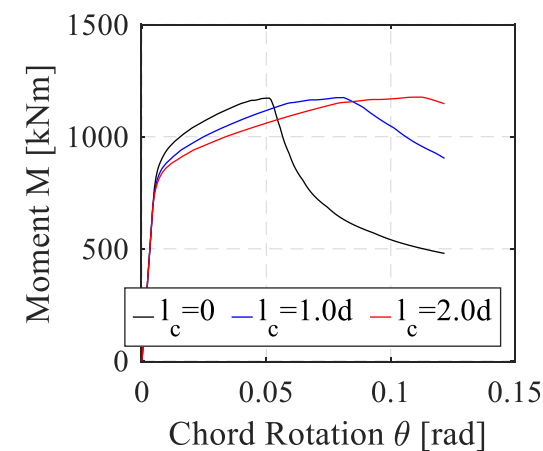
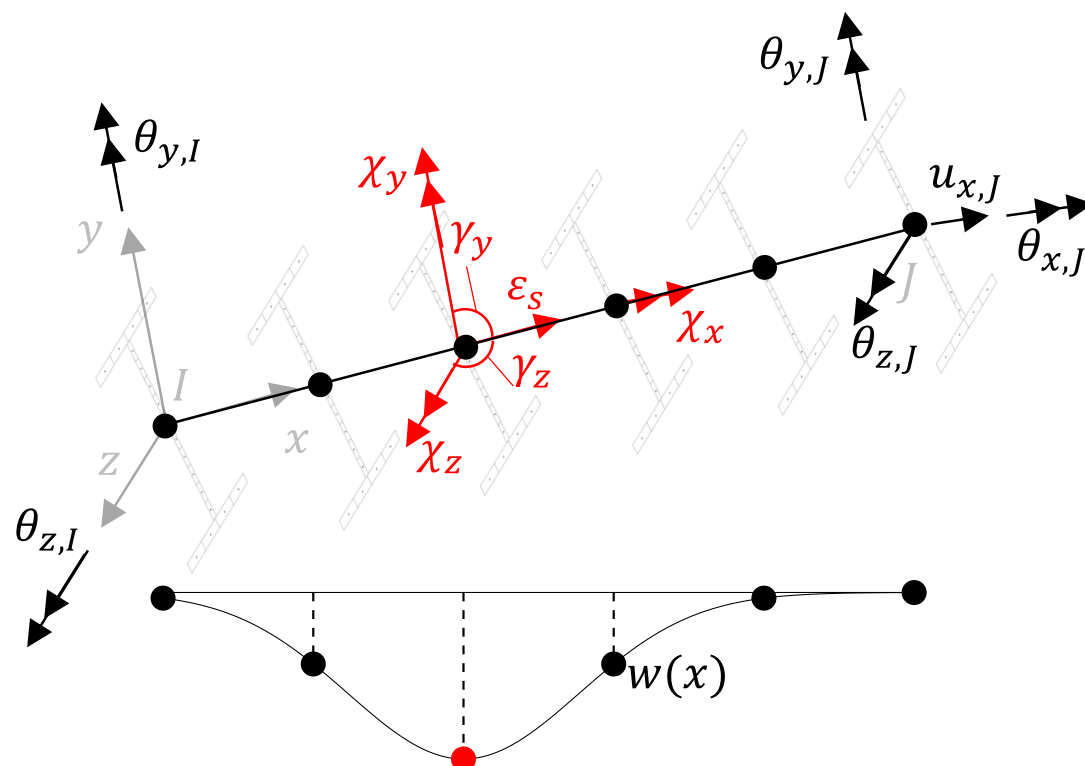
Source: Heredia, de Castro et Sousa and Lignos (2024)

3-dimensional beam-column elements (2)



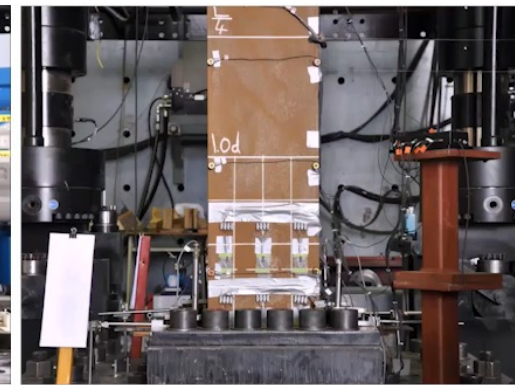
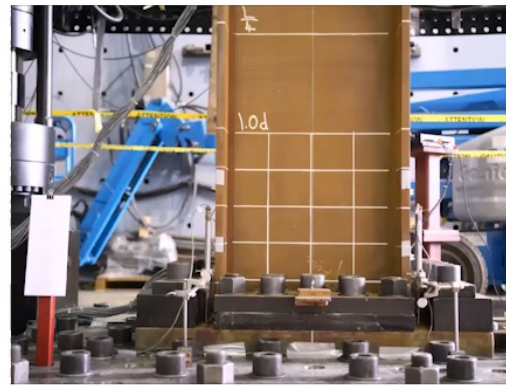
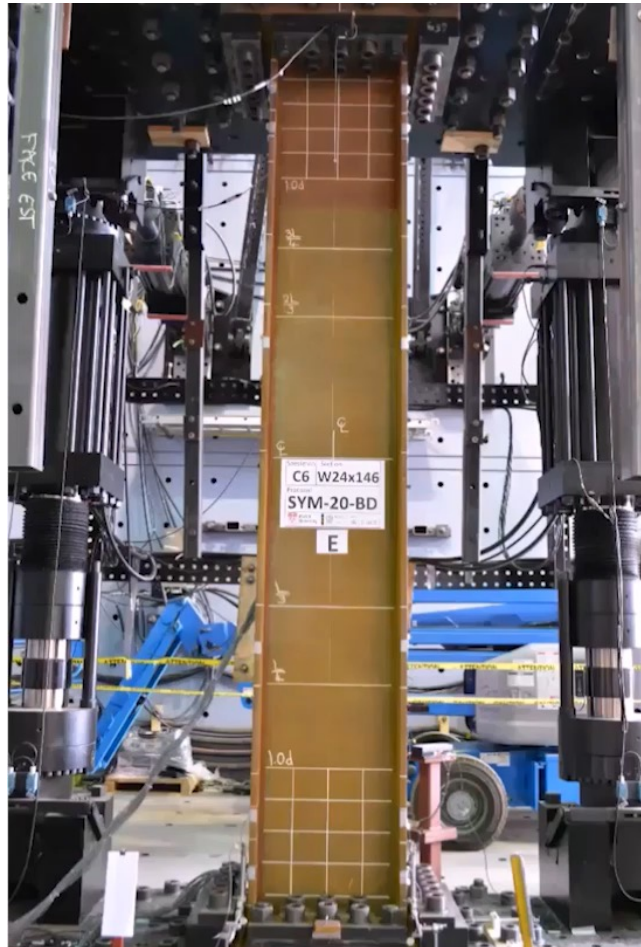
Source: Heredia, de Castro et Sousa and Lignos (2024)

3-dimensional beam-column elements (3)

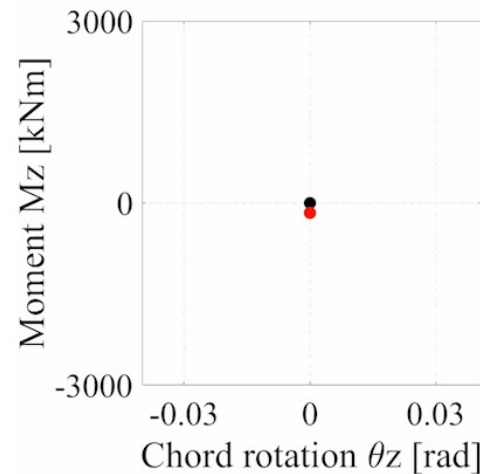


- Enhanced 3D Timoshenko force-based beam-column element (available in OpenSees)
- Implicit gradient nonlocal formulation

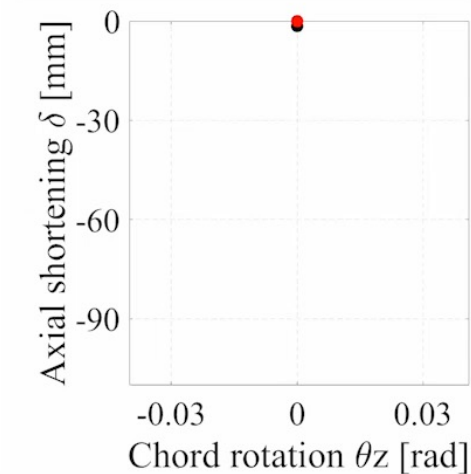
Source: Heredia, de Castro et Sousa and Lignos (2024)



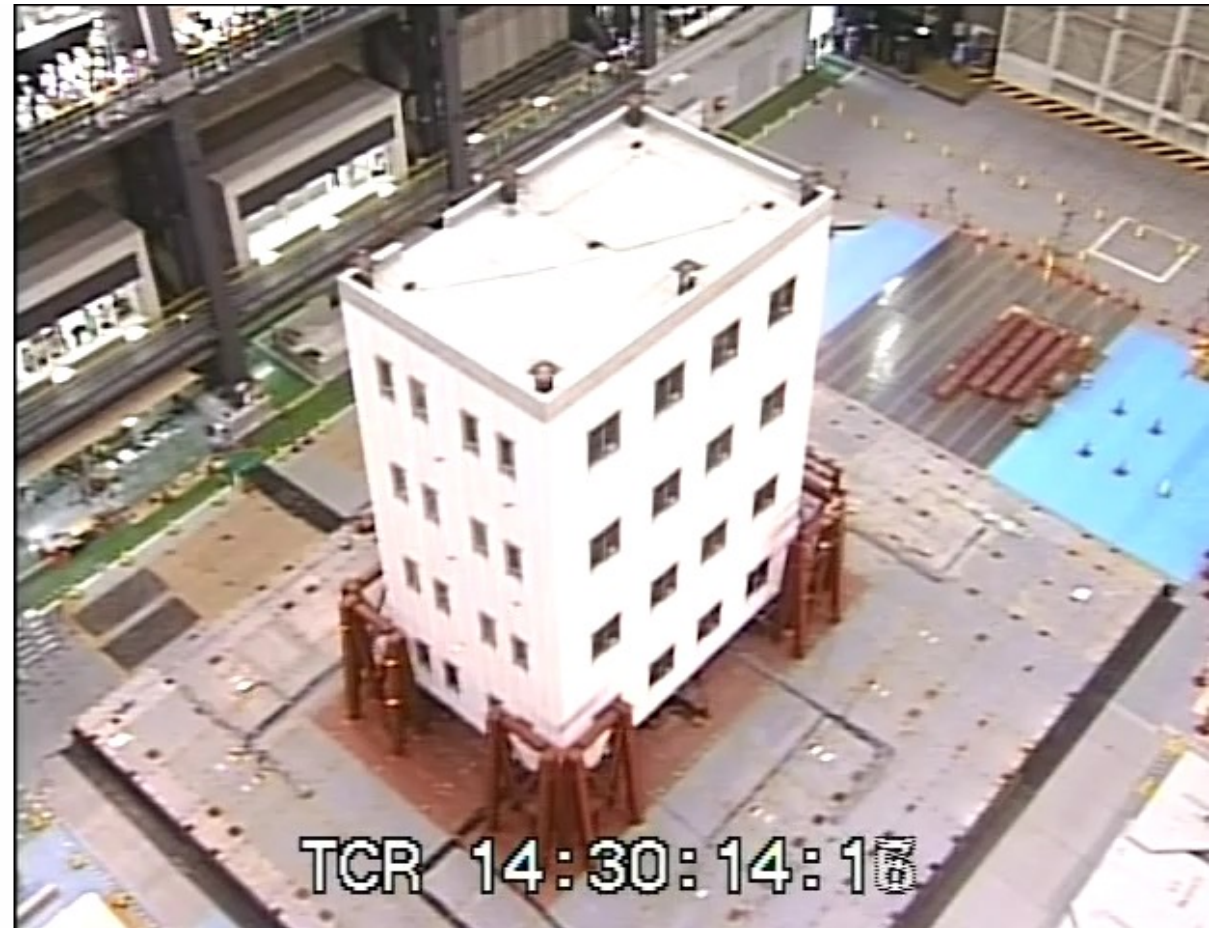
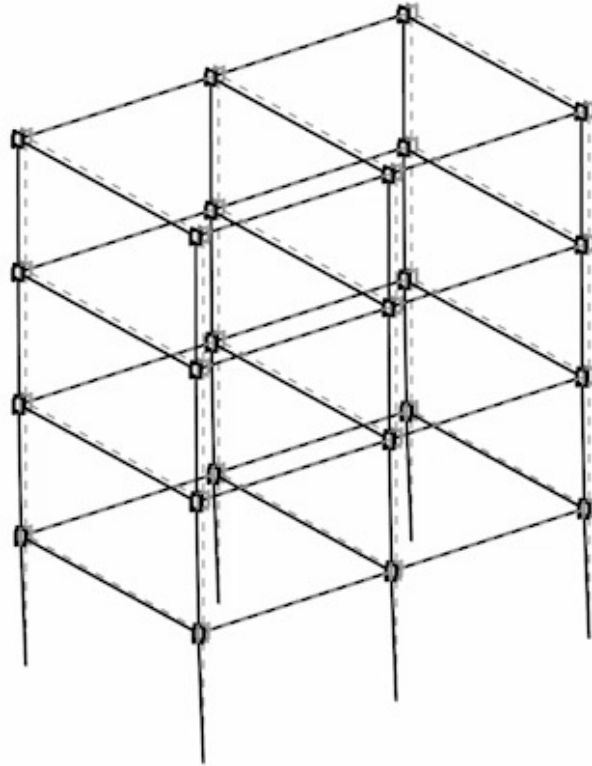
--- $n_{IP}=17$ --- $n_{IP}=21$ --- $n_{IP}=29$ — Test



--- $n_{IP}=17$ --- $n_{IP}=21$ --- $n_{IP}=29$ — Test



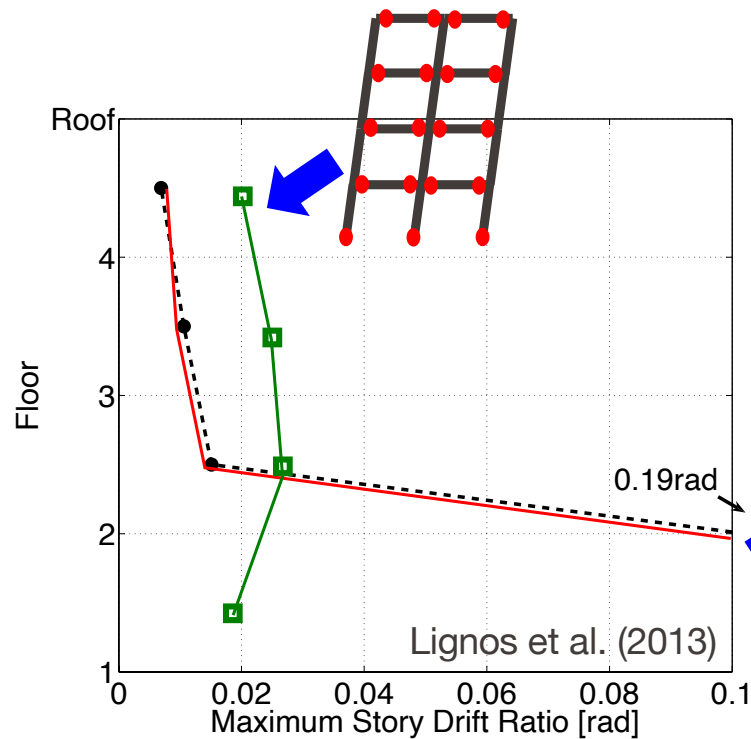
3-dimensional frame simulations to collapse



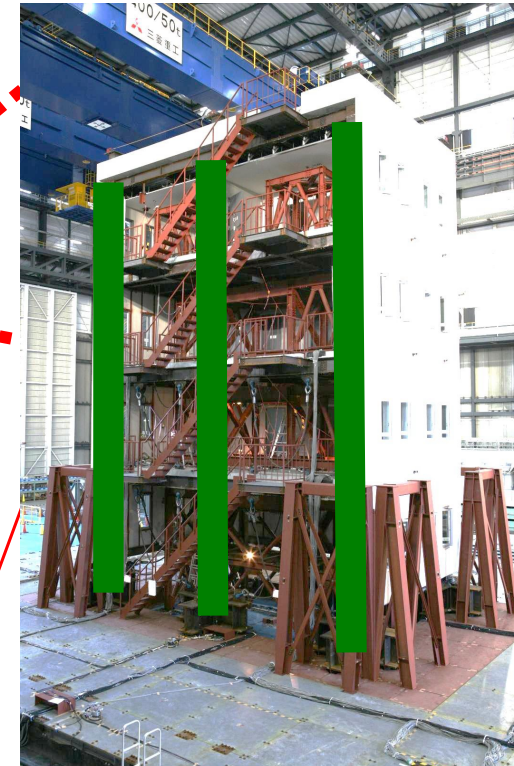
Source: Heredia, de Castro et Sousa and Lignos (2024)

Other benefits using nonlinear analysis

-Refinement of seismic standards



Collapse Mechanism



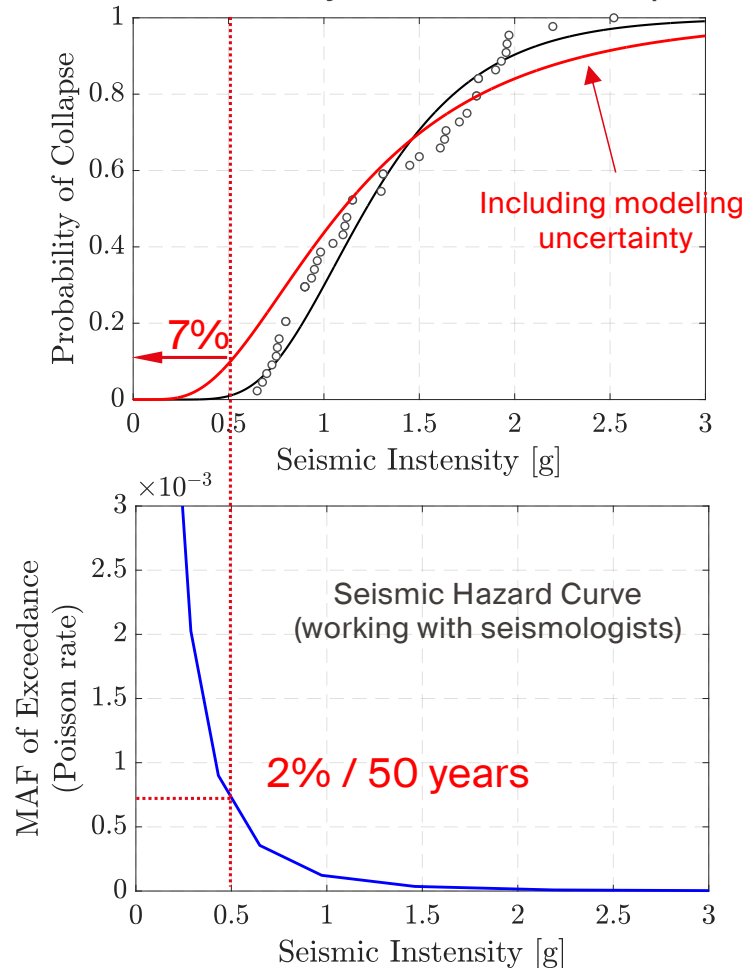
(Photo Source: E-Defense-2007)

-● Measured Data → 100% Intensity of JR Takatori Record
- Simulated Data (SCWB > 1.5) → 100% Intensity of JR Takatori Record
- Simulated Data (Proposed SCWB > 2.0) → 100% Intensity of JR Takatori Record

Source: Lignos et al. (2013)

EPFL Benchmarking building performance to extreme events

Vulnerability Curves for Collapse



Estimated Collapse Safety

- 7% probability of collapse under
“Maximum Considered Earthquake”
OR
- 0.7% Probability of collapse in 50 years

Question: Is this acceptable?

Perhaps...

In the meantime, such methodological developments can help improve our design standards in a systematic manner

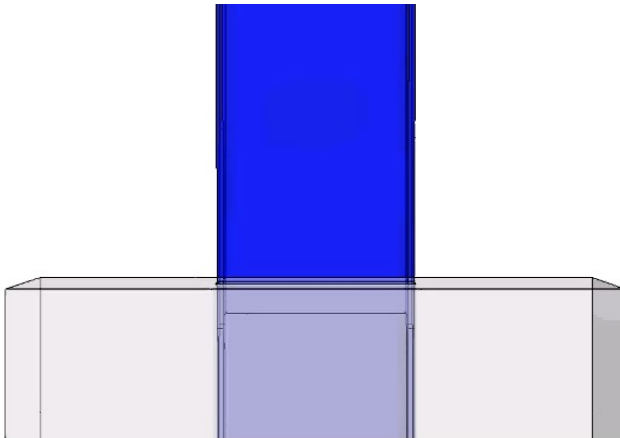
Elkady and Lignos (2015)

Eads, Miranda and Lignos (2016)

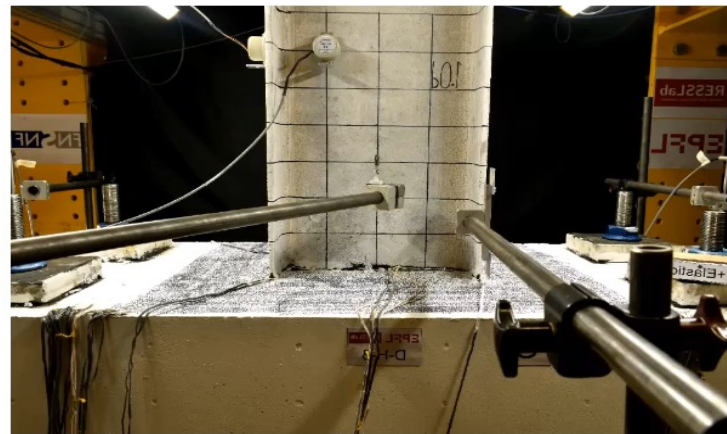
Other benefits using nonlinear analysis

-Steel columns resilient to earthquake damage

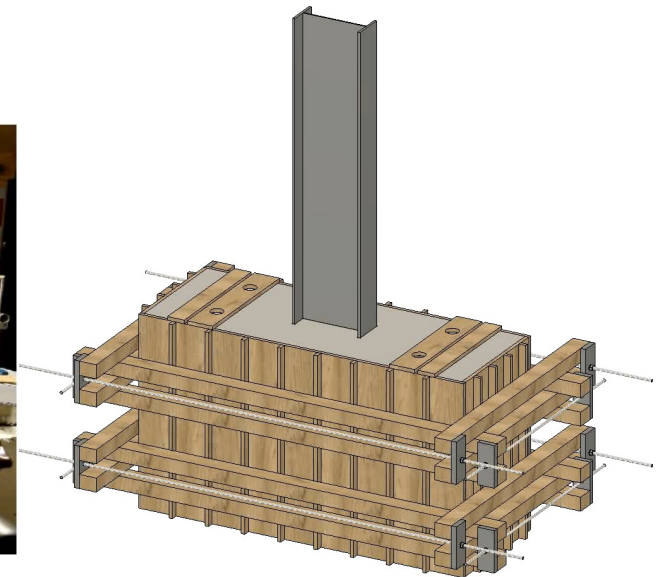
Conception assistée
par simulation



Validation du prototype
GIS @ EPFL



Séquence de fabrication



Inamasu, Sousa and Lignos (2022)

Other benefits using nonlinear analysis

Existing Medical Facilities 1960s



Photo courtesy of Prof. Robert Tremblay

Existing Tall Buildings (>40 stories) 1970s

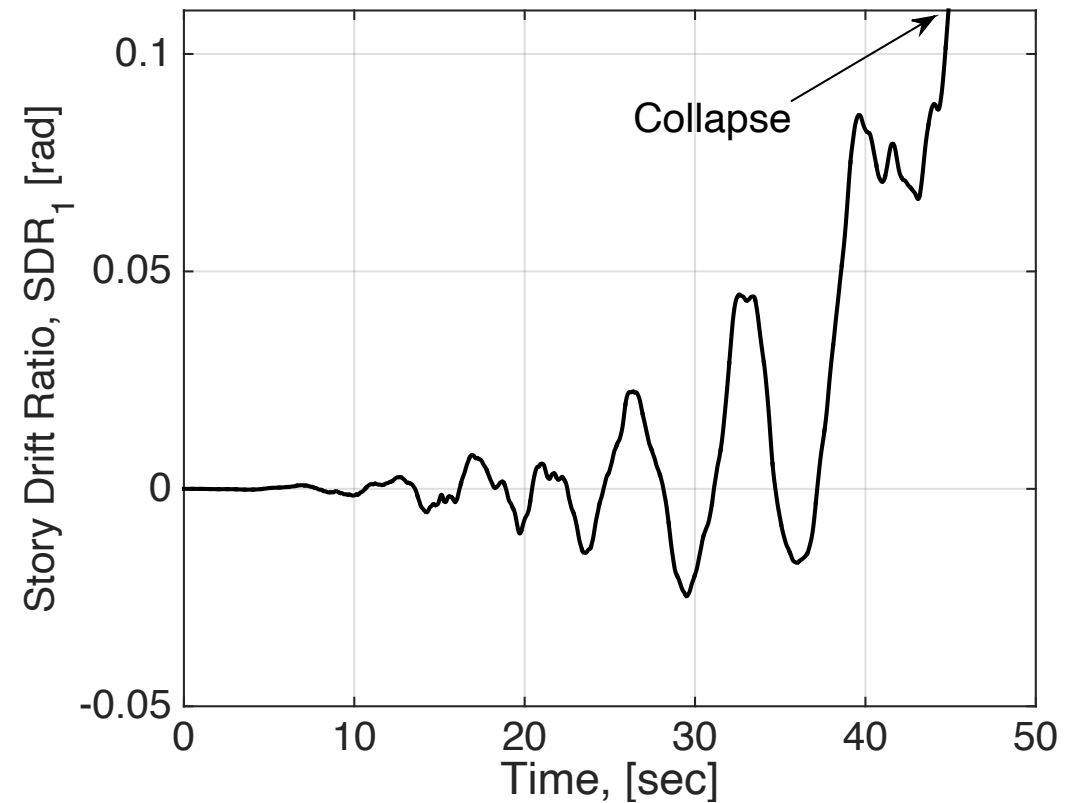
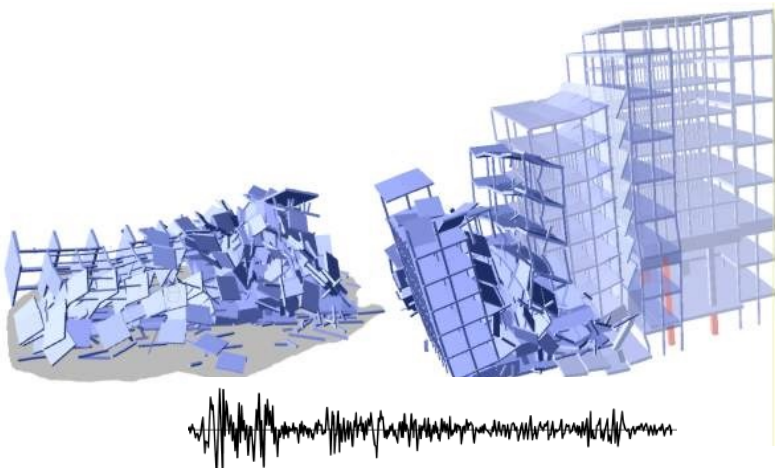


- ✧ Were built without seismic considerations (1960s – 1970s)
- ✧ Collapse risk is high in this case due to non-ductile failure modes

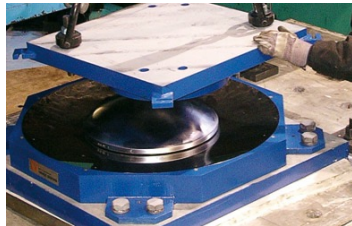
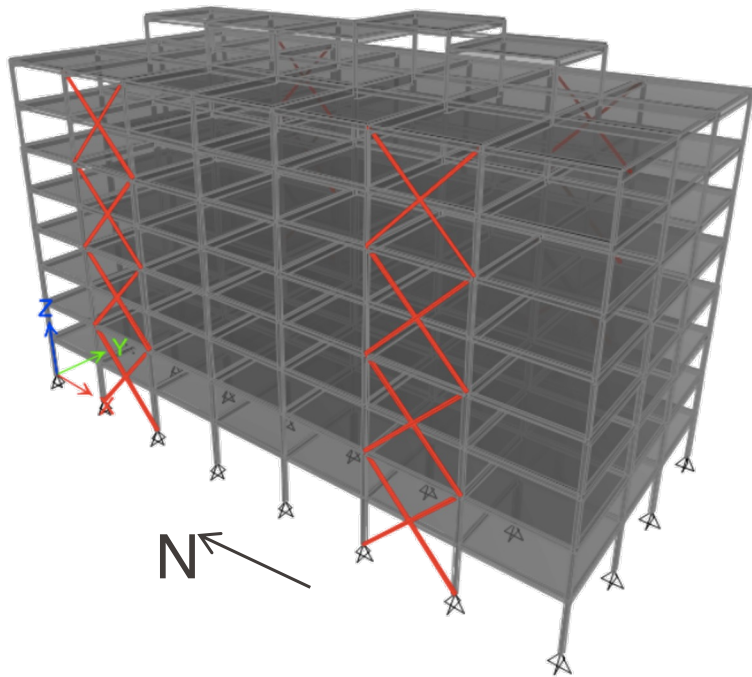
Other benefits using nonlinear analysis



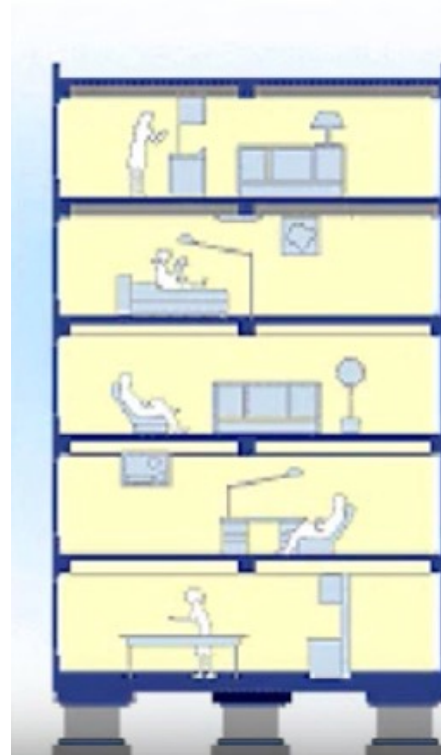
Nonlinear dynamic analysis simulation realization



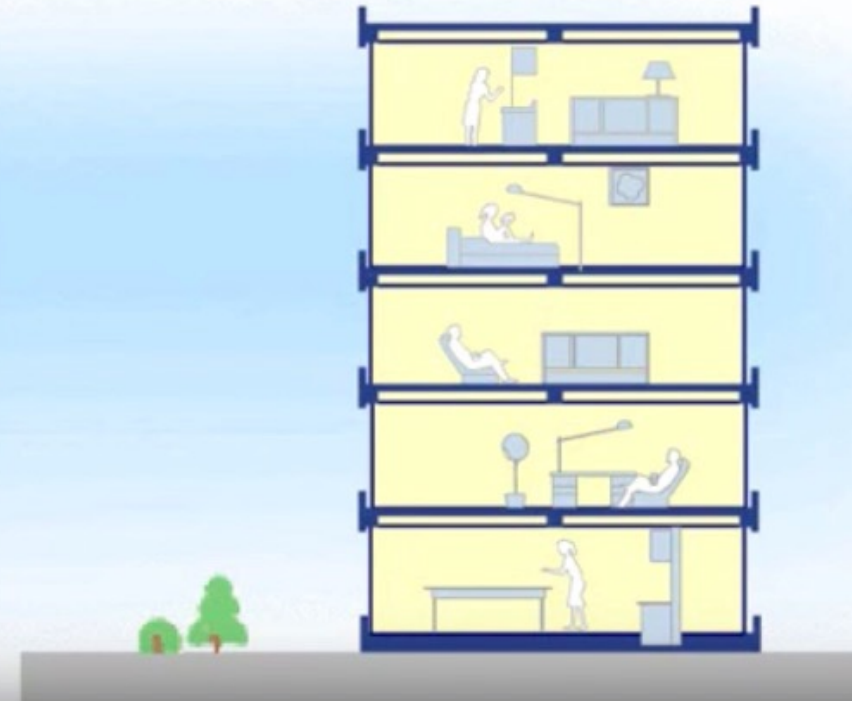
Other benefits using nonlinear analysis



Use of seismic isolator



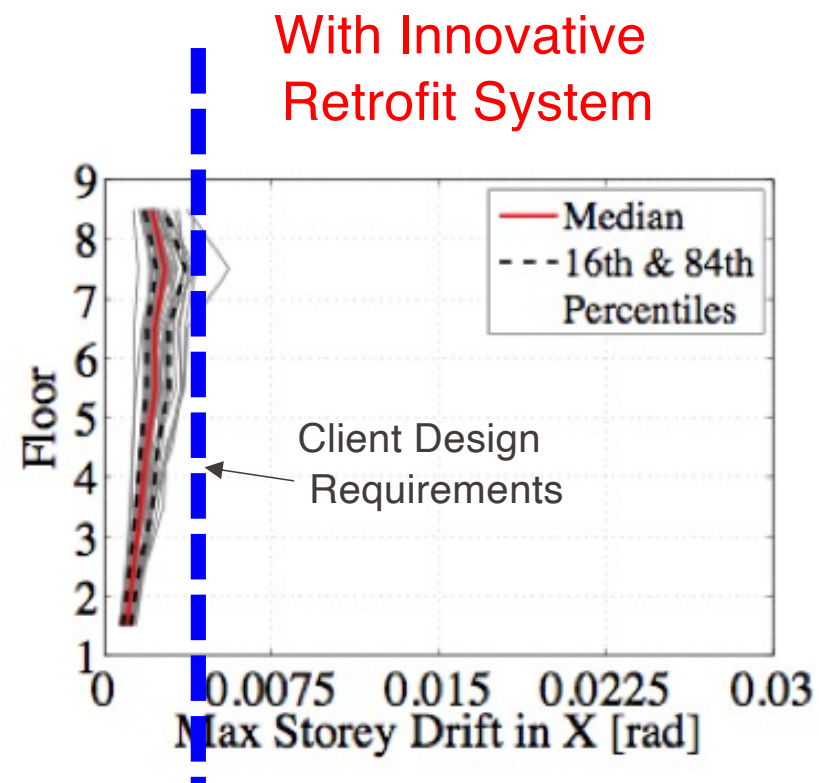
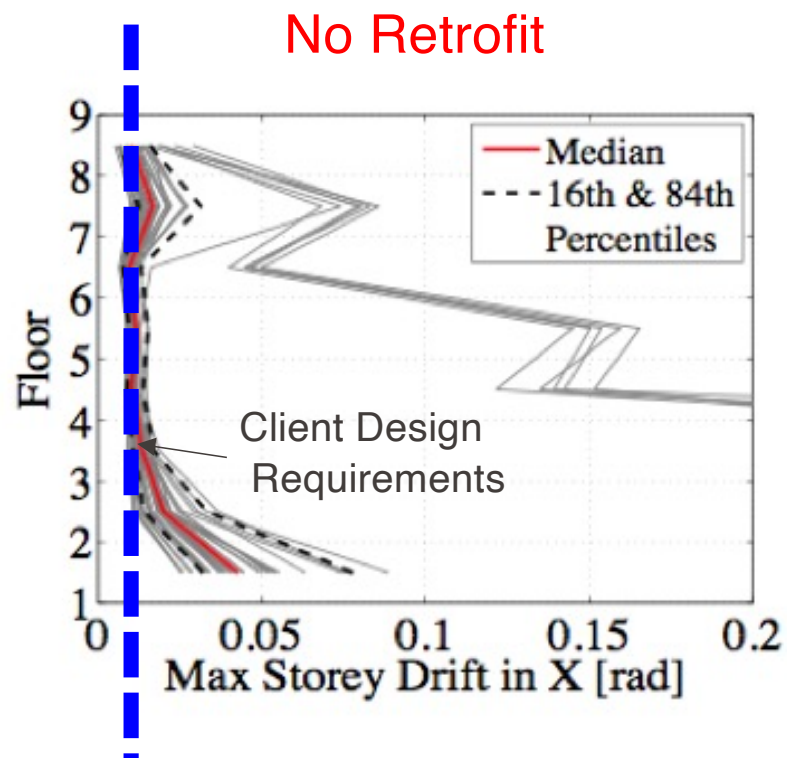
With isolation device



Without isolation device

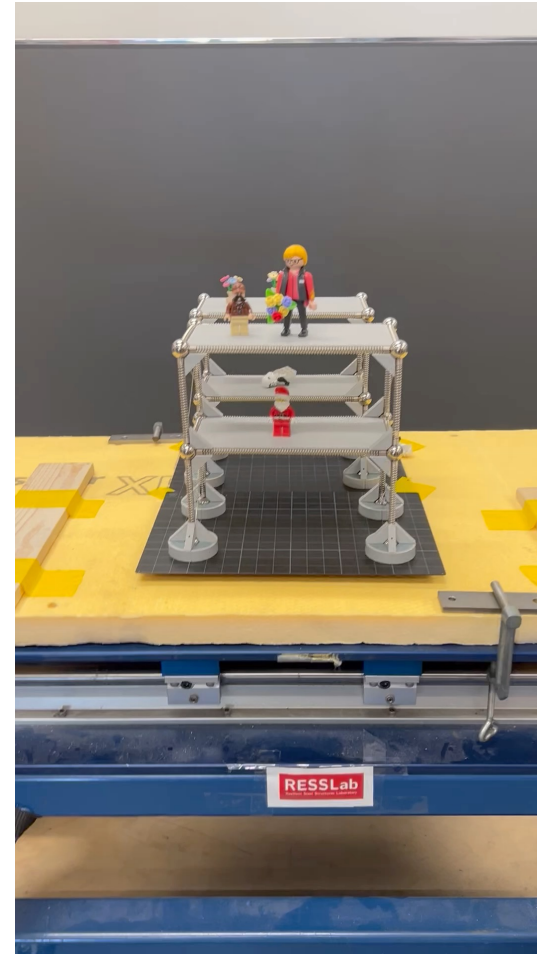
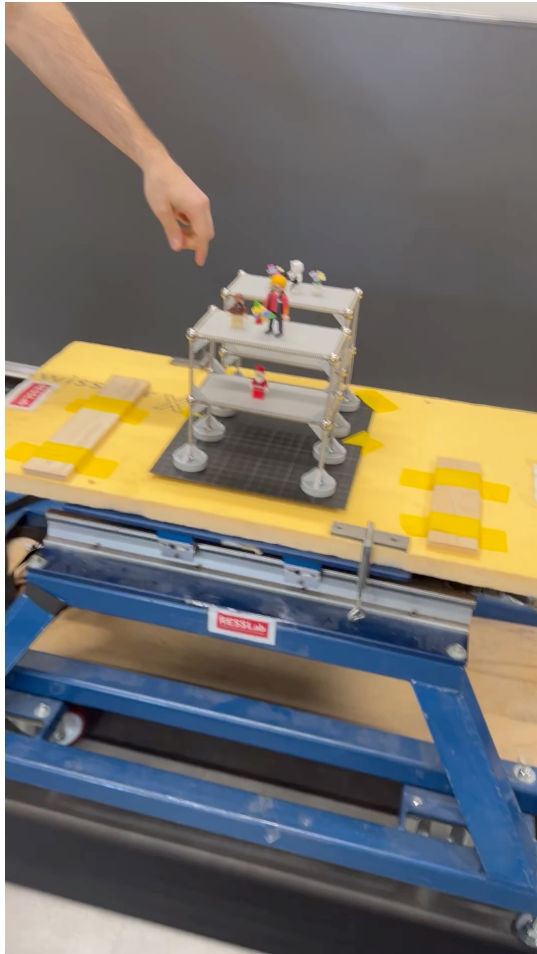
Source: Walker and Lignos (2015)*

Other benefits using nonlinear analysis



Source: Walker and Lignos (2015)*

Other benefits using nonlinear analysis



EPFL Some personal remarks

- A more complex model does not necessarily mean it is a better model.
 - The quote “Everything should be made as simple as possible but not simpler (A. Einstein)” does apply in nonlinear analysis
- Errors often occur in the post-processing. Engineering intuition is essential in this case to trace fundamental errors
- Validate your nonlinear model with available (and meaningful) measurements from prior test data and or analytical solutions of known problems.
- Other aspects (e.g., role of slab, nonstructural elements & many others) may be important considerations for a rational nonlinear structural model

EPFL Things we did not talk about

- Nonlinear dynamics
 - Mass & damping matrices
 - Algorithms for nonlinear dynamics
 - Rate effects (e.g., impact, explosions)
 - Rate-dependent versus rate independent constitutive formulations
- Contact problems
- Time-dependent and temperature-dependent constitutive formulations
- 3-dimensional effects
- Come for at least one semester project in nonlinear analysis (with RESSLab 😊)!

Thank you!