



# Hydraulic Engineering and Infrastructures

Civil Engineering Department

Hydraulic Structures

## 1 Free and submerged flow through a vertical sluice gate

We will use the notation and theoretical background presented in the *Gates and Hydraulic Jumps* notes available on Moodle.

This exercise concerns flow under a vertical sluice gate in a horizontal, rectangular channel of width  $b$ . Upstream of the gate the water depth is uniform and given by  $y_1$ . The gate opening is  $a$ , and due to contraction the jet thickness at the vena contracta is

$$y_g = C_c a,$$

where  $C_c$  is the contraction coefficient. The downstream uniform depth is denoted  $y_d$  (tailwater depth). We consider two operating regimes: free flow and submerged flow.

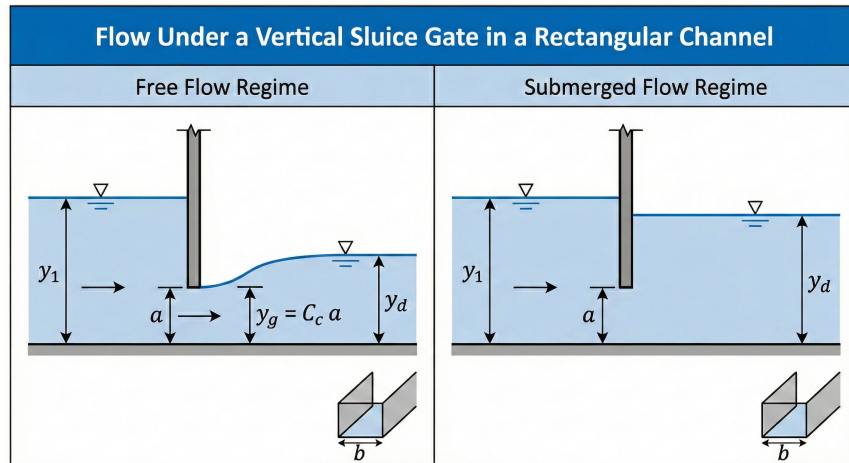


Figure 1: Free and submerged flow under a vertical sluice gate, showing upstream depth  $y_1$ , gate opening  $a$ , contracted jet thickness  $y_g = C_c a$ , and tailwater depth  $y_d$ .

Consider a sluice gate with geometric opening  $a = 0.10$  m in a rectangular channel of width  $b = 1.5$  m. Upstream depth is  $y_1 = 1.0$  m, and the contraction coefficient is  $C_c = 0.61$ . Take  $g = 9.81$  m/s<sup>2</sup>.

- (i) Using conservation of specific energy between the upstream section and the vena contracta, show that the free-flow discharge under the gate may be expressed as

$$Q_{\text{free}} = C_d b a \sqrt{2gy_1}, \quad \text{with} \quad C_d = \frac{C_c}{\sqrt{1 + \frac{C_c a}{y_1}}}.$$

Start by writing the specific energy at the upstream depth  $y_1$  and at the contracted jet thickness  $y_g = C_c a$ , then set  $E_1 = E_g$  to eliminate  $q$ . Show that this leads to the expression above for the discharge coefficient  $C_d$ . Finally, evaluate  $y_g$  and  $C_d$  numerically for the given data.

- (ii) Compute the free discharge  $Q_{\text{free}}$  through the gate.
- (iii) Determine the tailwater depth  $y_{d,\text{crit}}$  for which the hydraulic jump would attach to the gate and compare it with  $y_1$ .

## 2 Ogee Spillway Design

A high overflow spillway with  $P/H_d > 1.5$  has a maximum discharge of  $283.2 \text{ m}^3/\text{s}$  with a maximum expected head of  $6.10 \text{ m}$ . Using the USBR recommendation for the under-design procedure mentioned in class, determine the design head, spillway crest length (neglect contractions), and the minimum pressure (expressed in kPa) on the spillway. Plot the complete spillway crest shape for a compound circular curve in the upstream quadrant of the crest.