

Irrigation and Drainage Engineering

(Soil Water Regime Management)

(ENV-549)

4ETCS, Master option

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Platform of Hydraulic Constructions



Project: flooding in
Nouakchott (Mauritania) –
Groundwater modelling and
numerical solutions for steady
state

Water budget

- Input:
 - Precipitation
 - Groundwater recharge = definition
 - Injection
- Output:
 - Evapotranspiration
 - Pumping
 - Groundwater flow

Groundwater flow

- Groundwater = continuous variable (a value at a time depends on the value at the previous time)
- Determination of groundwater flow by measuring groundwater depth in observation wells (single point measurement)
- From groundwater depth measurements → piezometric map
- Groundwater flow from the highest hydraulic heads to the lowest → diffusivity equation

Diffusion equation

- Continuity equation

- 2D situation $\rightarrow w = 0$
- Steady state $\rightarrow \frac{\partial}{\partial t} = 0$

$$\frac{\partial \rho}{\partial t} + \rho \nabla \cdot \vec{U} = 0$$

$$\vec{U} = (u, v, w)$$

$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} = 0$$

- Darcy equation

$$\vec{U}_i = -K_i \frac{dh}{di}$$

$$\frac{\partial}{\partial x} \left(-K_x \frac{\partial h}{\partial x} \right) + \frac{\partial}{\partial y} \left(-K_y \frac{\partial h}{\partial y} \right) = 0$$

Directionally homogeneous and isotropic

$$K_x \frac{\partial^2 h}{\partial x^2} + K_y \frac{\partial^2 h}{\partial y^2} = 0$$

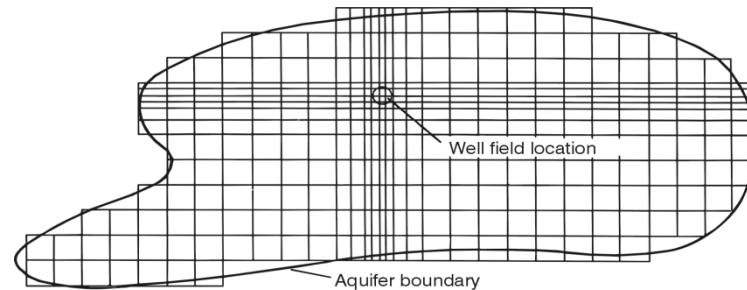
Fully homogeneous and isotropic

$$\frac{\partial^2 h}{\partial x^2} + \frac{\partial^2 h}{\partial y^2} = \nabla^2 h = 0$$

Laplace equation

Numerical modelling

- Simplifying assumptions for the diffusion equation
- Spatial discretization (cells of a grid) and temporal discretization (time steps)
- ➔ Finite difference to estimate the partial derivative in each cell (one equation per cell)



Fetter, 1994

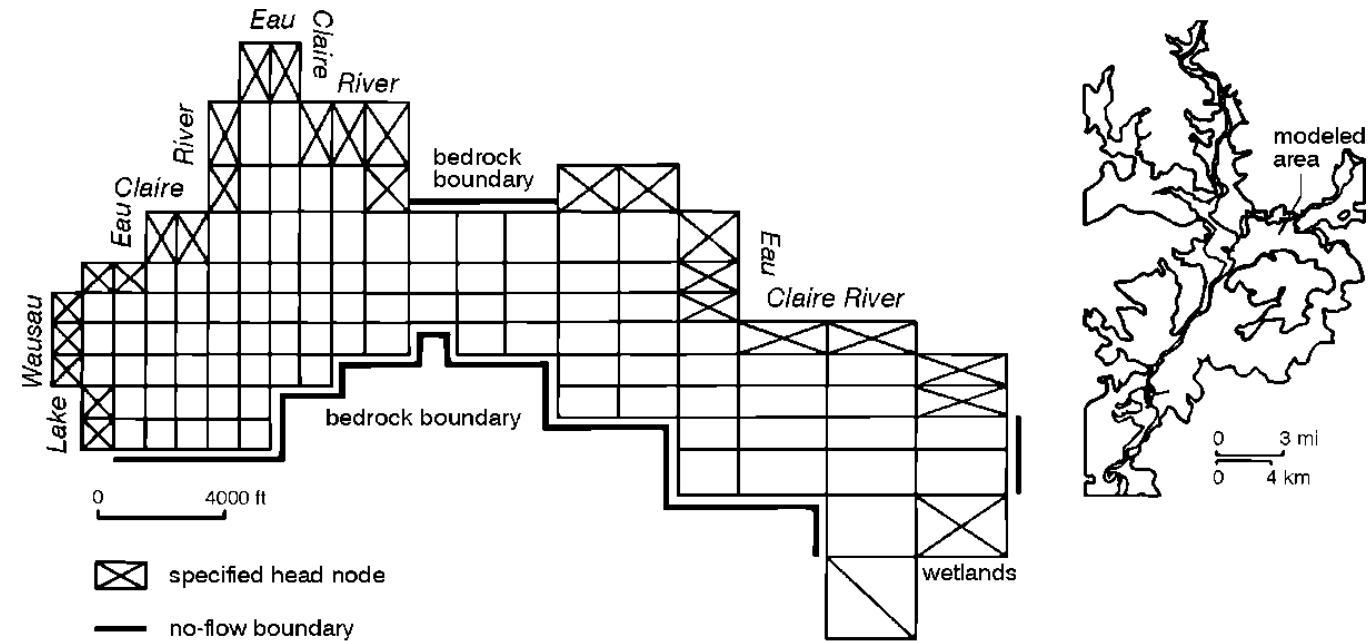
More on numerical solutions vs analytical solutions?

➔ Bear, J., & Cheng, A. H.-D. (2010). Modeling Groundwater Flow and Contaminant Transport. Springer Netherlands.

<https://doi.org/10.1007/978-1-4020-6682-5>

Needed information for simulation

- Hydraulic conductivity (or transmissivity) in each cell
- Pumping and groundwater recharge in each cell
- Aquifer geometry and spatial discretization
- Boundary conditions (constant head; specified flow, including no flow)



Anderson et Woessner, 1991

Steady state

- Finite difference solution

Conditions of application: unconfined water table, homogeneous, isotropic,
steady state, horizontal two-direction flow

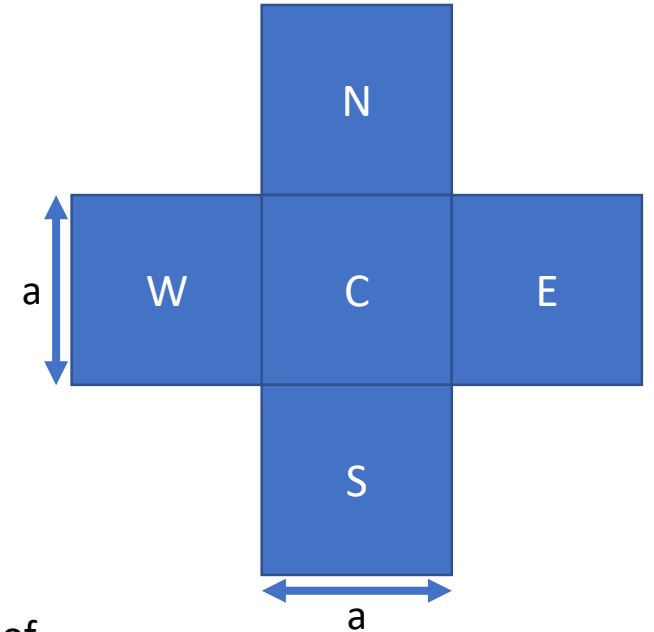
→ Darcy law on a square-shaped grid

→ Water budget for each cell of the grid

$$Q = Kab \frac{\Delta h}{L} \quad \text{so} \quad Q = Ta \frac{\Delta h}{L}$$

$$\begin{aligned} \sum Q_i &= 0 \\ \updownarrow L=a \\ \sum [T_i \Delta h_i] &= 0 \end{aligned}$$

With b the aquifer thickness
L the distance between the center of
two cells



Steady state

As the cells are square shaped:

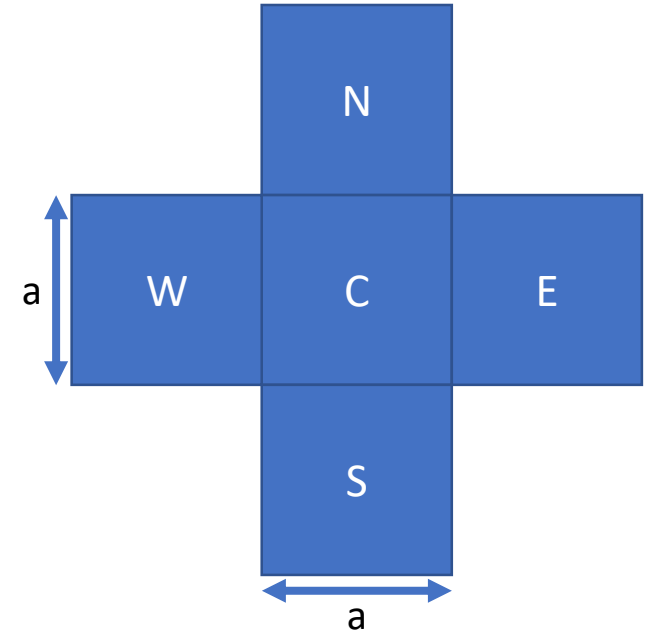
$$\sum [T_i \Delta h_i] = 0$$

$$T_{CN}(h_C - h_N) + T_{CS}(h_C - h_S) + T_{CE}(h_C - h_E) + T_{CW}(h_C - h_W) = 0$$

$$h_C = \frac{[T_{CN}h_N + T_{CS}h_S + T_{CE}h_E + T_{CW}h_W]}{[T_{CN} + T_{CS} + T_{CE} + T_{CW}]}$$

$$h_C = \frac{\sum [T_{iC}h_i]}{[\sum T_{iC}]}$$

→ As $T = \text{constant}$
$$h_C = \frac{T \sum [h_i]}{4T} = \frac{\sum [h_i]}{4}$$



Steady state

→ If well in cell C (pumping < 0, injection > 0)

$$\sum_i Q = -Q_p$$

$$\sum [T_i \Delta h_i] = -Q_p$$

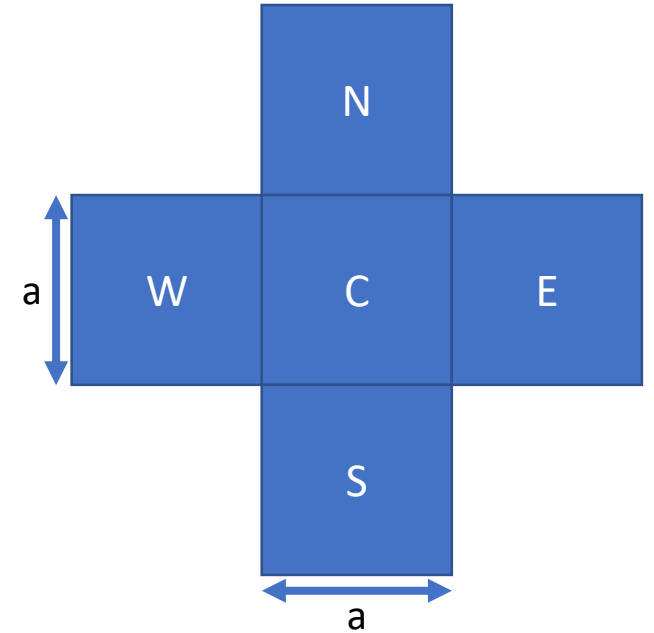
$$h_C = \frac{\sum [T_{iC} h_i] + Q_p}{\sum T_{iC}}$$

→ If groundwater recharge in cell C

$$\sum_i Q = w$$

$$\sum [T_i \Delta h_i] = w$$

$$h_C = \frac{\sum [T_{iC} h_i] - w}{\sum T_{iC}}$$



Steady state

- In Excel??

→ One equation for each cell

→ System with N equations

→ Iterative computing

More on groundwater modelling?

→ Anderson, M. P., Woessner, W. W., & Hunt, R. J. (2015). Applied Groundwater Modeling: Simulation of Flow and Advective Transport (Second Edition). Academic Press, Elsevier Inc.

Sheet 1: T

 T₁
 T₂

Sheet 2: groundwater recharge

W ₂	W ₂	W ₂	W ₂
W ₁	W ₂	W ₂	W ₂
W ₁	W ₁	W ₁	W ₂
W ₁	W ₁	W ₁	W ₁

Sheet 3: Pumping

	Q ₁	Q ₂	
		Q ₃	

Sheet 4: heads and equations

h=100					h=70
h=100					h=70
h=100					h=70
h=100					h=70

$$h_c = \frac{\sum [T_{ic} h_i] - w + Q_p}{\sum T_{ic}}$$

Constant head

No flow

Model precision

What do we use to compute the precision of a simulation?

→ The comparison of the simulated and observed variable(s) of interest

Which data for the Nouakchott project?

→ Simulated hydraulic head (groundwater level) and measured hydraulic heads

→ Simulated hydraulic head = 1 value for each cell of the simulated area (hydraulic head constant within each cell)

→ Measured hydraulic head = 3 observation wells + 1



Objective functions

Objective functions → indication on the performance of the model

« how well did the model perform? »

→ Comparison between simulated and observed variables

$$ME = \frac{1}{n} \sum_{t=1}^n (\text{sim}_t - \text{obs}_t)$$

With
n the number of measurements
Sim the simulated value
Obs the observed value

$$AME = \frac{1}{n} \sum_{t=1}^n |\text{sim}_t - \text{obs}_t|$$

An objective function is optimized ⇔ maximization or minimization depending on the formula

Name	Description	Formula*
DRMS	Daily Root Mean Squared Error	$\sqrt{\frac{1}{n} \sum_{t=1}^n (d_t - o_t(\theta))^2}$
TMVOL	Total Mean Monthly Volume Squared Error	$\sum_{i=1}^{n_{\text{month}}} \left(\frac{1}{n_{\text{day}}(i)} \sum_{t=1}^{n_{\text{day}}(i)} (d_t - o_t(\theta)) \right)^2$
ABSERR	Mean Absolute Error	$\frac{1}{n} \sum_{t=1}^n d_t - o_t(\theta) $
ABSMAX	Maximum Absolute Error	$\max_{1 \leq t \leq n} d_t - o_t(\theta) $
NS	Nash-Sutcliffe Measure	$1 - \frac{\frac{1}{n} \sum_{t=1}^n [d_t - o_t(\theta)]^2}{\frac{1}{n} \sum_{t=1}^n (d_t - \bar{d})^2}$
BIAS	Bias (mean daily error)	$\frac{1}{n} \sum_{t=1}^n (d_t - o_t(\theta))$
PDIFF	Peak Difference	$\max_{1 \leq t \leq n} \{d_t\} - \max_{1 \leq t \leq n} \{o_t(\theta)\}$
RCOEF	First Lag Autocorrelation	$\frac{\frac{1}{n} \sum_{t=1}^n (d_t - o_t(\theta))(d_{t+1} - o_{t+1}(\theta))}{\sigma_d \sigma_{o(\theta)}}$
NSC	Number of Sign Changes	(Count the number of times the sequence of residuals changes sign)

*Minimize with respect to θ .

Vijai Gupta et al. (1998)

Darcy law

→ Calculate the groundwater flux between two points

In the Nouakchott model, as the cells are square shaped:

$$Q = Kab \frac{\Delta h}{L}$$

$$Q = Ta \frac{\Delta h}{L}$$

With K the hydraulic conductivity (m.s^{-1})

a the side of the cell (m)

b the thickness of the aquifer (m)

Δh the difference in hydraulic head between the center of 2 cells

L the distance between the center of 2 cells (m)

T the transmissivity ($\text{m}^2.\text{s}^{-1}$) ($T = K*b$)

