

CIVIL 449: Nonlinear Analysis of Structures

School of Architecture, Civil & Environmental Engineering
Civil Engineering Institute

Enforcement of Constraints

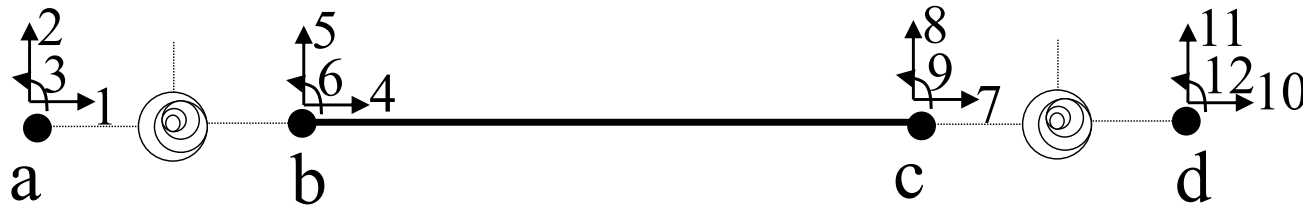
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EPFL, ENAC, IIC, RESSLab

EPFL Objectives of today's lecture

- To introduce:
 - How to apply constraints in finite element models

EPFL Motivation

- The figure below shows an elastic beam-column element with two rotational springs at the element ends



- As seen in Week#3, nodes a and b have the same coordinates (same for nodes c and d)
- The left and right springs act between degrees of freedom 3 and 6 (left spring) and 9 and 12 (right spring)
- Additional constraints must be defined between degrees of freedom 1,2,4 and 5 (left spring) and 7,8,10 and 11 (right spring) to ensure that the springs remain at zero length

EPFL Additional constraints

- The following constraints are added to the model:

$$\text{Left spring:} \begin{cases} v_1 = v_4 \\ v_2 = v_5 \end{cases} \text{ and right spring:} \begin{cases} v_7 = v_{10} \\ v_8 = v_{11} \end{cases}$$

Where v_k denotes the displacement at the global degree of freedom k

EPFL Constraints enforcement – Transformation approach (1)

- Constraint equations that couple degrees of freedom in $\mathbf{v}$ can be written in the form

$$\mathbf{C} \cdot \mathbf{v} = \mathbf{Q}$$

Where $\mathbf{C}$ and $\mathbf{Q}$ contain constants.

- Consider the common case $\mathbf{Q} = \mathbf{0}$, the constraint equation is partitioned so that

$$[\mathbf{C}_r \quad \mathbf{C}_c] \begin{bmatrix} \mathbf{v}_r \\ \mathbf{v}_c \end{bmatrix} = \mathbf{0}$$

Where $\mathbf{v}_r$ and $\mathbf{v}_c$ are the dofs to be retained and dofs to be condensed out, respectively

- Because there are as many dofs $\mathbf{v}_c$ as there are independent equations of constraint, matrix $\mathbf{C}_c$ is square and nonsingular

EPFL Constraints enforcement – Transformation approach (2)

- Solving for $\mathbf{v}_c$ yields

$$\mathbf{v}_c = \mathbf{C}_{rc} \mathbf{v}_r \text{ where } \mathbf{C}_{rc} = -\mathbf{C}_c^{-1} \mathbf{C}_r$$

- This equation is combined with the identity $\mathbf{v}_r = \mathbf{v}_r$:

$$\begin{bmatrix} \mathbf{v}_r \\ \mathbf{v}_c \end{bmatrix} = \mathbf{T} \mathbf{v}_r \text{ where } \mathbf{T} = \begin{bmatrix} \mathbf{I} \\ \mathbf{C}_{rc} \end{bmatrix}$$

- The transformation $\mathbf{F} = \mathbf{T}^T \mathbf{F}'$ and $\mathbf{K} = \mathbf{T}^T \mathbf{K}' \mathbf{T}$ can be applied to the structural equation $\mathbf{F}' = \mathbf{K}' \mathbf{v}'$

EPFL Constraints enforcement – Transformation approach (3)

- Similarly to the condensation procedure presented in Week#2, the structural equations $\mathbf{F}' = \mathbf{K}'\mathbf{v}'$ can be partitioned as

$$\begin{bmatrix} \mathbf{K}_{rr} & \mathbf{K}_{rc} \\ \mathbf{K}_{cr} & \mathbf{K}_{cc} \end{bmatrix} \begin{bmatrix} \mathbf{v}_r \\ \mathbf{v}_c \end{bmatrix} = \begin{bmatrix} \mathbf{F}_r \\ \mathbf{F}_c \end{bmatrix}$$

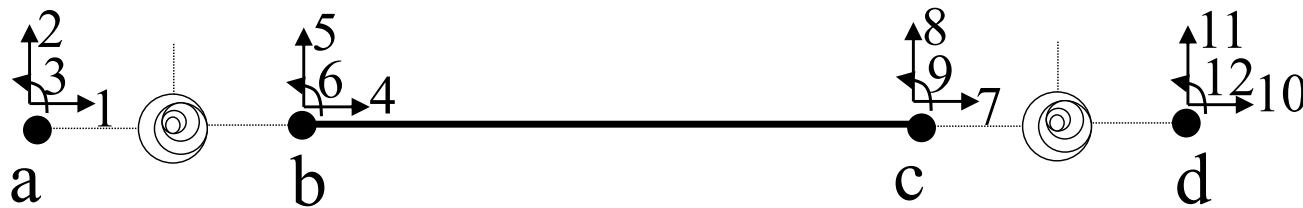
- The condensed system is

$$[\mathbf{K}_{rr} + \mathbf{K}_{rc}\mathbf{C}_{rc} + \mathbf{C}_{rc}^T\mathbf{K}_{cr} + \mathbf{C}_{rc}^T\mathbf{K}_{cc}\mathbf{C}_{rc}]\mathbf{v}_r = [\mathbf{F}_r + \mathbf{C}_{rc}^T\mathbf{F}_c]$$

- After this equation is solved for $\mathbf{v}_r$, the displacement corresponding to the condensed out degrees of freedom $\mathbf{v}_c$ can be computed using $\mathbf{v}_c = \mathbf{C}_{rc}\mathbf{v}_r$

Example: Beam element with two end springs (1)

- Consider the following structure consisting of a single 2D elastic beam-column element with two inelastic rotational spring at its ends



- Applying the transformation method, write the global structural equation

EPFL Example: Beam element with two end springs (2)

- As previously discussed, the following constraints are imposed:

$$\text{Left spring: } \begin{cases} v_1 = v_4 \\ v_2 = v_5 \end{cases} \text{ and right spring: } \begin{cases} v_7 = v_{10} \\ v_8 = v_{11} \end{cases}$$

- The condensed degrees of freedom are selected as v_1, v_2, v_{10} and v_{11}
- The constrained equations are given by

$$[C_r \quad C_c] \begin{bmatrix} v_r \\ v_c \end{bmatrix} = \mathbf{0} \rightarrow \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 & | & -1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & | & 0 & -1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 & | & 0 & 0 & -1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 & | & 0 & 0 & 0 & -1 \end{bmatrix} \begin{bmatrix} v_3 \\ v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \\ v_{12} \\ - \\ v_1 \\ v_2 \\ v_{10} \\ v_{11} \end{bmatrix} = \mathbf{0}$$

Example: Beam element with two end springs (3)

- And

$$\mathbf{C}_{rc} = -\mathbf{C}_c^{-1}\mathbf{C}_r = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

- The transformation matrix is given by

$$\mathbf{T} = \begin{bmatrix} \mathbf{I} \\ \mathbf{C}_{rc} \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

EPFL Example: Beam element with two end springs (4)

- The force-displacement relations are given by
 - For the left spring (spring stiffness $k_{s1} = a_1 EI_e/L$):

$$\begin{bmatrix} F_3 \\ F_6 \end{bmatrix} = \begin{bmatrix} k_{s1} & -k_{s1} \\ -k_{s1} & k_{s1} \end{bmatrix} \begin{bmatrix} v_3 \\ v_6 \end{bmatrix}$$

- For the right spring (spring stiffness $k_{s2} = a_2 EI_e/L$):

$$\begin{bmatrix} F_9 \\ F_{12} \end{bmatrix} = \begin{bmatrix} k_{s2} & -k_{s2} \\ -k_{s2} & k_{s2} \end{bmatrix} \begin{bmatrix} v_9 \\ v_{12} \end{bmatrix}$$

Example: Beam element with two end springs (5)

- For the elastic beam-column element:

$$\begin{Bmatrix} F_4 \\ F_5 \\ F_6 \\ F_7 \\ F_8 \\ F_9 \end{Bmatrix} = \frac{E}{L} \begin{bmatrix} A & 0 & 0 & -A & 0 & 0 \\ 0 & aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & -aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) \\ 0 & aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & aI_e 4 \left(1 + \frac{3}{a_2}\right) & 0 & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 2aI_e \\ -A & 0 & 0 & A & 0 & 0 \\ 0 & -aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) \\ 0 & aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 2aI_e & 0 & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & aI_e 4 \left(1 + \frac{3}{a_1}\right) \end{bmatrix} \begin{Bmatrix} v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \end{Bmatrix}$$

Example: Beam element with two end springs (6)

- The complete system is given by:

$$\begin{Bmatrix} F_1 \\ F_2 \\ F_3 \\ F_4 \\ F_5 \\ F_6 \\ F_7 \\ F_8 \\ F_9 \\ F_{10} \\ F_{11} \\ F_{12} \end{Bmatrix} = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & k_{s1} & 0 & 0 & -k_{s1} & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & \frac{AE}{L} & 0 & 0 & -\frac{AE}{L} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 & 0 & 0 \\ 0 & 0 & -k_{s1} & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_2}\right) + k_{s1} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 2 \frac{aEI_e}{L} & 0 & 0 & 0 \\ 0 & 0 & 0 & -\frac{AE}{L} & 0 & 0 & \frac{AE}{L} & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 2 \frac{aEI_e}{L} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_1}\right) + k_{s2} & 0 & 0 & -k_{s2} \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & -k_{s2} & 0 & 0 & k_{s2} \end{bmatrix} \begin{Bmatrix} v_1 \\ v_2 \\ v_3 \\ v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \\ v_{10} \\ v_{11} \\ v_{12} \end{Bmatrix}$$

EPFL Example: Beam element with two end springs (7)

- The reduced system is given by

$$\mathbf{F} = \mathbf{K}\mathbf{v}_r \rightarrow \mathbf{T}^T \mathbf{F}' = \mathbf{T}^T \mathbf{K}' \mathbf{T} \mathbf{v}_r$$

- Which gives

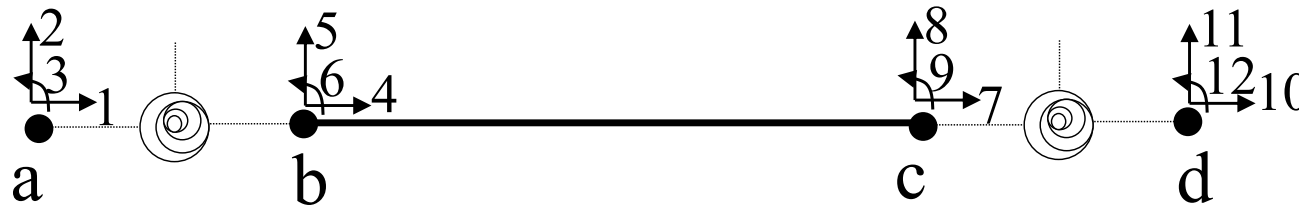
$$\begin{bmatrix} F_3 \\ F_4 + F_1 \\ F_5 + F_2 \\ F_6 \\ F_7 + F_{10} \\ F_8 + F \\ F_9 \\ F_{12} \end{bmatrix} = \begin{bmatrix} k_{s1} & 0 & 0 & -k_{s1} & 0 & 0 & 0 & 0 \\ 0 & \frac{AE}{L} & 0 & 0 & -\frac{AE}{L} & 0 & 0 & 0 \\ 0 & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 \\ -k_{s1} & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_2}\right) + k_{s1} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 2 \frac{aEI_e}{L} & 0 \\ 0 & -\frac{AE}{L} & 0 & 0 & \frac{AE}{L} & 0 & 0 & 0 \\ 0 & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 \\ 0 & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 2 \frac{aEI_e}{L} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_1}\right) + k_{s2} & -k_{s2} \\ 0 & 0 & 0 & 0 & 0 & 0 & -k_{s2} & k_{s2} \end{bmatrix} \begin{bmatrix} v_3 \\ v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \\ v_{12} \end{bmatrix}$$

EPFL Constraints enforcement – Direct approach (1)

- Another approach to directly enforce constraint which impose **equal degrees of freedom** of the form $v_n = v_m$ (where v_m is to be condensed out), consists in assigning the same number to both degree of freedom number at both nodes.
- The external force acting on degrees of freedom m must be transferred to the degree of freedom n
- It is important to note that this approach is **only applicable to impose equal degrees of freedom constraints** of the form $v_n = v_m$

EPFL Constraints enforcement – Direct approach (2)

- For example, consider the previous example:



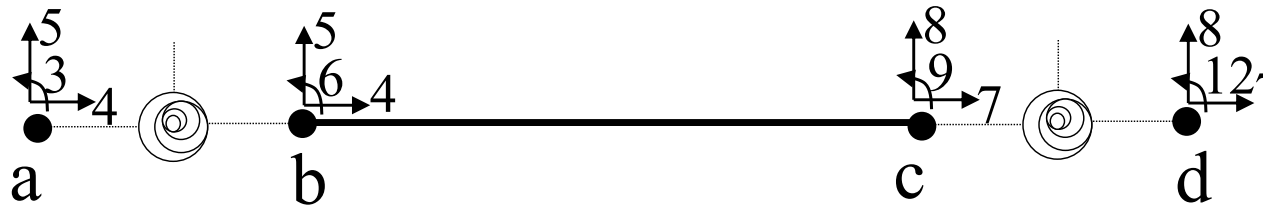
- As previously discussed, the following constraints are imposed:

$$\text{Left spring: } \begin{cases} v_1 = v_4 \\ v_2 = v_5 \end{cases} \text{ and right spring: } \begin{cases} v_7 = v_{10} \\ v_8 = v_{11} \end{cases}$$

- The condensed degrees of freedom are selected as v_1, v_2, v_{10} and v_{11}

EPFL Constraints enforcement – Direct approach (3)

- The degrees of freedom are transformed as follows:



- With these degrees of freedom, the force-displacement relations are given by

- For the left spring (spring stiffness $k_{s1} = a_1 EI_e / L$):

$$\begin{bmatrix} F_3 \\ F_6 \end{bmatrix} = \begin{bmatrix} k_{s1} & -k_{s1} \\ -k_{s1} & k_{s1} \end{bmatrix} \begin{bmatrix} v_3 \\ v_6 \end{bmatrix}$$

- For the right spring (spring stiffness $k_{s2} = a_2 EI_e / L$):

$$\begin{bmatrix} F_9 \\ F_{12} \end{bmatrix} = \begin{bmatrix} k_{s2} & -k_{s2} \\ -k_{s2} & k_{s2} \end{bmatrix} \begin{bmatrix} v_9 \\ v_{12} \end{bmatrix}$$

Constraints enforcement – Direct approach (4)

- For the elastic beam-column element:

$$\begin{Bmatrix} F_4 \\ F_5 \\ F_6 \\ F_7 \\ F_8 \\ F_9 \end{Bmatrix} = \frac{E}{L} \begin{bmatrix} A & 0 & 0 & -A & 0 & 0 \\ 0 & aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & -aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) \\ 0 & aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & aI_e 4 \left(1 + \frac{3}{a_2}\right) & 0 & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 2aI_e \\ -A & 0 & 0 & A & 0 & 0 \\ 0 & -aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & aI_e \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) \\ 0 & aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 2aI_e & 0 & -aI_e \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & aI_e 4 \left(1 + \frac{3}{a_1}\right) \end{bmatrix} \begin{Bmatrix} v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \end{Bmatrix}$$

Constraints enforcement – Direct approach (5)

- Assembling the global force-deformation relation directly gives

$$\begin{bmatrix} F_3 \\ F_4 + F_1 \\ F_5 + F_2 \\ F_6 \\ F_7 + F_{10} \\ F_8 + F \\ F_9 \\ F_{12} \end{bmatrix} = \begin{bmatrix} k_{s1} & 0 & 0 & -k_{s1} & 0 & 0 & 0 & 0 \\ 0 & \frac{AE}{L} & 0 & 0 & -\frac{AE}{L} & 0 & 0 & 0 \\ 0 & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 \\ -k_{s1} & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_2}\right) + k_{s1} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 2 \frac{aEI_e}{L} & 0 \\ 0 & -\frac{AE}{L} & 0 & 0 & \frac{AE}{L} & 0 & 0 & 0 \\ 0 & 0 & -\frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_2}\right) & 0 & \frac{aEI_e}{L} \frac{12}{L^2} \left(1 + \frac{a_1 + a_2}{a_1 a_2}\right) & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 0 \\ 0 & 0 & \frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & 2 \frac{aEI_e}{L} & 0 & -\frac{aEI_e}{L} \frac{6}{L} \left(1 + \frac{2}{a_1}\right) & \frac{aEI_e}{L} 4 \left(1 + \frac{3}{a_1}\right) + k_{s2} & -k_{s2} \\ 0 & 0 & 0 & 0 & 0 & 0 & -k_{s2} & k_{s2} \end{bmatrix} \begin{bmatrix} v_3 \\ v_4 \\ v_5 \\ v_6 \\ v_7 \\ v_8 \\ v_9 \\ v_{12} \end{bmatrix}$$

- Which corresponds to the system obtained with the constraint transformation approach