Traffic Engineering (CIVIL-349) Nikolas Geroliminis Exercise 9 Traffic Signal Design Fall 2024

## Step 1: Signal phasing selection

Observing the topology of the intersection, one can note that there is an exclusive lane for a protected left turn coming from the West. Moreover, it shows that, in all other approaches, the channelization does not give exclusive lanes for turning. At the same time, there is no approach coming from the North.

Table 1 shows the volumes per lane of each movement.

Movement	$\mathrm{O} \to \mathrm{D}$	Lanes	Total Volume	Volume per lane	Table 1:
1	$W \to E$	2	1000	500	•
2	$W \to N$	1	250	250	
3	$\mathrm{E}  ightarrow (\mathrm{W,N})$	2	900 + 250	575	
4	$S \rightarrow (W,N,E)$	3	70 + 1100 + 80	$416.ar{6}$	

To define the best method for phasing, we will test three methods: Approach, Protected left turn, and Overlap (although not introduced in class, this refers to a phasing where a particular movement may be included in two consecutive phases).

Signal Phases	Movements	Volumes	Critical Volume
$\phi_1$	4	$416.\bar{6}$	$416.ar{6}$
$\phi_2$	1,2	500,500,250	500
$\phi_3$	3	575,575	575
Total			1491.ē

Signa	l Phases	Movements	Volumes	Critical Volume
	$\phi_1$	$4^1$	$416.\bar{6}$	$416.ar{6}$
	$\phi_2$	1,3	500,500,575,575	575
	$\phi_3$	2	250	250
Т	otal			1241.ē

Note that the method with the worst total critical volume is the Approach method; this will in turn lead to the longest cycle time, and is therefore less desirable. At the same time, there is a tie between the Protected Left turn and Overlap methods. For demonstration, we will proceed with the phases for the overlap method. The figure below

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Table 1: List of volumes per movement.

Table 2: Approach method: critical volumes.

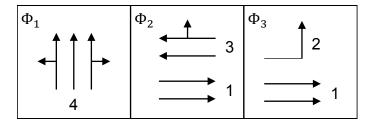
Table 3: Protected left turn method: critical volumes.

<sup>1</sup> Note that we did not separate the left turn here, because there is no conflicting flow coming from the North.

Signal Phases	Movements	Volumes	Critical Volume
$\phi_1$	4	$416.\overline{6}$	$416.\overline{6}$
$\phi_2$	3,(1)	(500),(500),575,575	575
$\phi_3$	(1),2	250,(500),(500)	250
Total			1241.ē

Table 4: Overlap method: critical volumes.

summarizes the choice for the phasing step, where the movement labels from Table 1 are indicated.



Step 2: Determine cycle time

The optimal cycle duration can be computed using Webster Formula.

$$C_o = \frac{1.5 \times L + 5}{1 - \sum_i y_i} \tag{1}$$

The lost time is per phase is given (4 seconds). The only remaining variables to compute are the flow/saturation flow rate for each phase. Table 5 computes the remaining variables.

Phase	Critical flow	Saturation flow	$y_i$
$\phi_1$	$416.ar{6}$	1800	0.2315
$\phi_2$	575	1800	0.3194
$\phi_3$	250	1800	0.1389

Table 5: Computation of flow/saturation rates for each phase  $(y_i)$ .

Hence, the optimal cycle duration is (rounded to the next 5 seconds):

$$C_o = \frac{1.5 \times 4 \times 3 + 5}{1 - (0.2315 + 0.3194 + 0.1389)} = 74.146 \approx 75$$
seconds

Step 3: Determine green allocation

Using the information from the previous steps, we can use the following formula to compute the effective green time allocation of each phase.

$$g_i = \frac{y_i}{\sum_i y_i} \times (C_o - L) \tag{2}$$

Phase	$y_i$	$g_i$ (rounded)
$\phi_1$	0.2315	21 sec
$\phi_2$	0.3194	29 sec
$\phi_3$	0.1389	13 sec

Table 6: Computation of the green time allocation for each phase  $(g_i)$ .

As these numbers are rounded, it is interesting to verify whether there was a change in the total green time ( $C_0 - L$ ).

$$C_o - L = 75 - 3 \times 4 = 63 = \sum_i g_i = 21 + 29 + 13 = 63$$
 ok

Step 4: Intergreen time allocation

Step 4.1: Yellow time allocation

Yellow time is defined as:

$$y = T + \frac{v}{2a + 2Gg} \tag{3}$$

Variable	Description	Value
$\overline{T}$	Driver reaction time	1.4 seconds
v	Speed of the vehicle	$40 \text{mph} \approx 58.67 \text{ft/s}$
а	Deceleration rate speed	$11 \text{ft/s}^2$
G	Inclination (slope)	0

Table 7: Variable description for yellow time computation.

Note that yellow time is the same for all phases. The final value is:

$$y = T + \frac{v}{2a + 2Gg} = 1.4 + \frac{58.67}{2 \times 11} = 4.07 \rightarrow 4.5 \text{ seconds}$$
 (4)

Step 4.2: All red time allocation

We can compute 'All red time' using the following equation:

$$AR = \frac{w+l}{v} \tag{5}$$

Variable	Description	Value
$\overline{w}$	Width of the intersection	12ft/lane and 10ft/crosswalk
1	Length of one vehicle	20ft
v	Speed of the vehicle	$40$ mph $\approx 58.67$ ft/s

Table 8: Variable description for 'all red' time computation.

As the number of lanes is different in both direction, width of the road (w) is also different. Two different all red times are computed:

Phase  $\Phi_1$ : Vehicles have to cross 5 lanes and 2 crosswalks:

$$AR_1 = \frac{5 \times 12 + 2 \times 10 + 20}{58.67} = 1.7 \rightarrow 2 \text{ seconds}$$
 (6)

Phase  $\Phi_2$  and phase  $\Phi_3$ : Vehicles have to cross 3 lanes and 2 crosswalks:

$$AR_{2,3} = \frac{3 \times 12 + 2 \times 10 + 20}{58.67} = 1.3 \rightarrow 1.5 \text{ seconds}$$
 (7)

## Summary:

	Phase 1	Phase 2-3
Yellow	4.5	4.5
All Red	2	1.5
Total lost time	6.5	6

Step 5: Pedestrian Requirement (minimum green time)

The minimum green time for pedestrians is computed using the following equation:

$$G_p = 3.2 + \frac{d}{v_{ped}} + 0.27 \times N_{ped}$$
 (8)

Variable	Description	Value
d	Length of the crosswalk (not width)	12ft/lane
$v_{ped}$	Speed of pedestrians	4ft/s
$N_{ped}$	Number of pedestrians per direction	5

Table 9: Variable description for the computation of the minimum green time for pedestrians.

Note that the length of the crosswalk varies depending on the road pedestrians have to cross.

During phase  $\phi_1$ , pedestrians are allowed to cross in the North-South (and South-North) direction, i.e., they have to cross 5 lanes.

$$G_{p1} = 3.2 + \frac{5 \times 12}{4} + 0.27 \times 5 = 19.55 \approx 20 \text{ seconds}$$
 (9)

To verify whether this is enough time, one must compare with the effective green time of the respective phase. Note that the time needed must be smaller than the effective green time.

$$G_{n1} < g_1 \to 20 < 21 \text{ ok}$$
 (10)

Pedestrians can cross in the West-East (or East-West) direction during both phases  $\phi_2$  and  $\phi_3$ .

$$G_{p2,3} = 3.2 + \frac{3 \times 12}{4} + 0.27 \times 5 = 13.55 \rightarrow 14 \text{ seconds}$$
 (11)

$$G_{p2,3} < g_2 + g_3 \rightarrow 14 < 29 + 13 = 42 \text{ ok}$$
 (12)

## Step 6: Signal Timing

Now that we have all the parameters for the traffic light at the intersection, we need to assign the proper times for each one of the phases (green, yellow, and all red) in order. The first step for this is to compute the actual time that the signal will display the green light (taking into account the will will lose some of the time for usual losses and the allocation of yellow and red lights).

Green display = Effective green + Lost time 
$$-Y - AR$$
 (13)

Phase 
$$\Phi_1$$
:  $G_1 = g_1 + L - Y - AR_1 = 21 + 4 - 4.5 - 2 = 18.5 s$   
Phase  $\Phi_2$ :  $G_2 = g_2 + L - Y - AR_2 = 29 + 4 - 4.5 - 1.5 = 27 s$   
Phase  $\Phi_3$ :  $G_3 = g_3 + L - Y - AR_3 = 13 + 4 - 4.5 - 1.5 = 11 s$ 

As a final step, we must verify whether any rounding we performed so far generated an error in the total cycle length.

$$G_1 + G_2 + G_3 + 3 \times Y + \sum_i AR_i = C_o$$
  
 $\Rightarrow 18.5 + 27 + 11 + 3 \times 4.5 + (2 + 1.5 + 1.5) =$   
 $= 75 \text{ s} \rightarrow \text{ok}$  (14)

Hence, the final traffic signal diagram becomes as shown below.

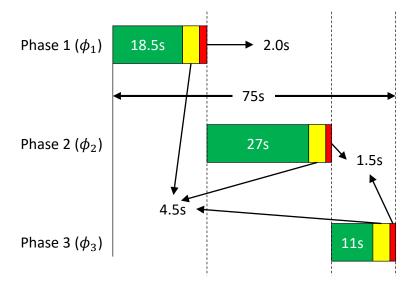


Figure 1: Final traffic signal diagram.